



SEAL

CONSULTANT

OWNER

LOCATION

DATE

SHEET

FLETCHER BRIGHT COMPANY

1007 ASHLAND TERRACE
 SUITE 104A
 CHATTANOOGA, TN 37415

DOLLAR TREE SHELL -
 McMINNVILLE

912 N CHANCERY ST
 McMINNVILLE, TN 37110
 WARREN COUNTY

PERMIT SET
05/16/24
 MJM # 23336

NO.	DESCRIPTION	DATE

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FLOOR PLAN

A100

GENERAL NOTES

- PRIME AND PAINT TO A LEVEL 4 FINISH ALL NEW GYP. BD. WALLS, CEILING AND BULKHEADS.
- ALL NEW CONCRETE SLABS AND REPAIR TRENCHES AT SLAB CUTS TO BE 4" THICK 3,000 PSI CONCRETE OVER COMPACTED STONE BASE ON 6 MIL VAPOR RETARDER.
- VERIFY ALL PARTITION LAYOUTS AND DIMENSIONS IN FIELD PRIOR TO BEGINNING ERECTION OF PARTITIONS. NOTIFY ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.
- DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE IN QUESTION OR IF THERE ARE CONFLICTS, CONTACT THE ARCHITECT FOR CLARIFICATION.
- ALL PARTITION DIMENSIONS SHOWN ARE NOMINAL FROM FINISH FACE TO FINISH FACE U.N.O.
- ALL NEW WALLS ARE 3-5/8" METAL STUDS @ 16" O.C. WITH 5/8" GYP. BD. BOTH SIDES U.N.O. STUD GAUGE AS RECOMMENDED BY MANUFACTURER OR 2012 SSMA TECHNICAL MANUAL FOR UNRACED HEIGHTS AT STUD SIZE AND SPACING INDICATED AT 5 PSF LATERAL LOAD AND 1/240 MAXIMUM DEFLECTION.
- PROVIDE STANDARD 5 LB ABC FIRE EXTINGUISHERS AT LOCATIONS AS DIRECTED BY THE FIRE MARSHAL.
- ALL CONCEALED WOOD, CONCEALED WOOD BLOCKING, PLY WOOD BACKING MATERIALS, ETC. SHALL BE FIRE RETARDANT TREATED WOOD HAVING A FLAME SPREAD OF 25 OR LESS.
- ALL FASTENERS USED WITH FIRE RETARDANT TREATED WOOD SHALL COMPLY W/ SECTION 2304.10.5.3 & 2304.10.5.4 OF THE 2015 IBC. FASTENERS SHALL BE OF HOT DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER.

LIFE SAFETY NOTES

- SEE ELECTRICAL DRAWING FOR LIFE SAFETY & EMERGENCY LIGHTING.
- SEE ELECTRICAL DRAWING FOR FIRE ALARM, HORN, STROBE, AND SMOKE DETECTORS.
- FIRE PROTECTION DESIGN TO BE SUBMITTED DURING CONSTRUCTION BY THE GENERAL CONTRACTOR IN THE FORM OF SHOP DRAWINGS FOR JURISDICTION APPROVAL.
- FINAL FIRE EXTINGUISHER LOCATIONS TO BE CONFIRMED BY LOCAL FIRE MARSHAL & TENANT DURING WALK THRU.
- FIRE EXTINGUISHER SIZE AND PLACEMENT FOR CLASS "A" HAZARDS:
 * 11,250 SF MAX. FLOOR AREA FOR EACH FIRE EXTINGUISHER
 * 75' MAX. TRAVEL DISTANCE TO AN EXTINGUISHER
 * 1,500 SF PER UNIT OF TYPE "A"
 * MINIMUM RATED EXTINGUISHER - 2A10B.C
- FIRE EXTINGUISHER SIZE AND PLACEMENT FOR CLASS "B" HAZARDS:
 * ORDINARY (MODERATE) HAZARD - SIZE 10B EXTINGUISHER RATING
 * 30'0" MAX. TRAVEL DISTANCE TO EXTINGUISHER
 * SIZE 20B EXTINGUISHER RATING REQUIRES 50'0" MAX. TRAVEL DISTANCE TO EXTINGUISHER.
- FIRE EXTINGUISHER SIZE AND PLACEMENT FOR CLASS "C" HAZARDS:
 * REQUIRED WHERE ENERGIZED ELECTRICAL EQUIPMENT CAN BE ENCOUNTERED THAT WOULD REQUIRE A NON-CONDUCTING EXTINGUISHING MEDIUM.

WALL SCHEDULE

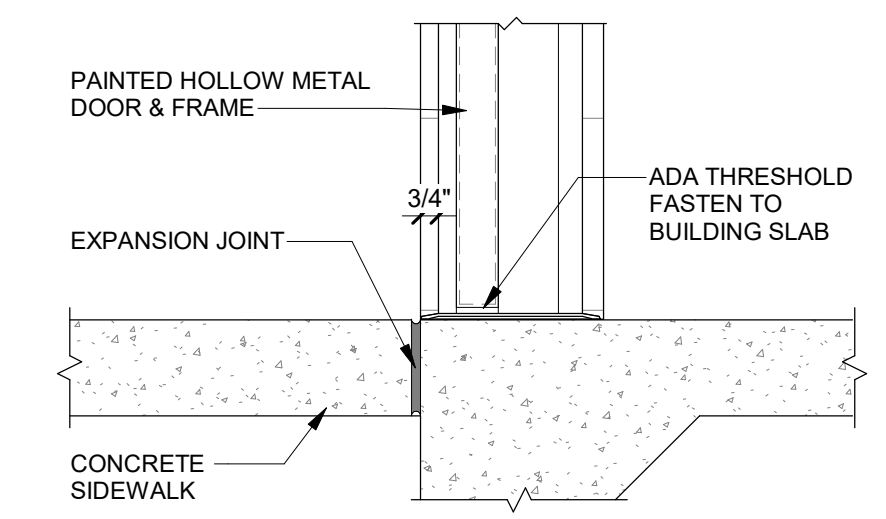
MARK	DESCRIPTION
1	4" SPLIT FACE CMU VENEER UP TO 2'-8" W/ 1-1/2" EIFS ABOVE OVER 5/8" EXT SHEATHING FASTENED TO 8" CMU
2	8" SMOOTH-FACE CMU W/ CORE-FILL INSULATION TO 6" ABOVE ROOF DECK (SEE STRUCTURAL FOR REINFORCING)
3	5/8" GYP BD OVER 3-5/8" METAL STUD FLOORING @ 16" OC W/ MIN R-13 BATT INSULATION OVER CONTINUOUS RIGID R-5 INSULATION

FLOOR TRANSITIONS

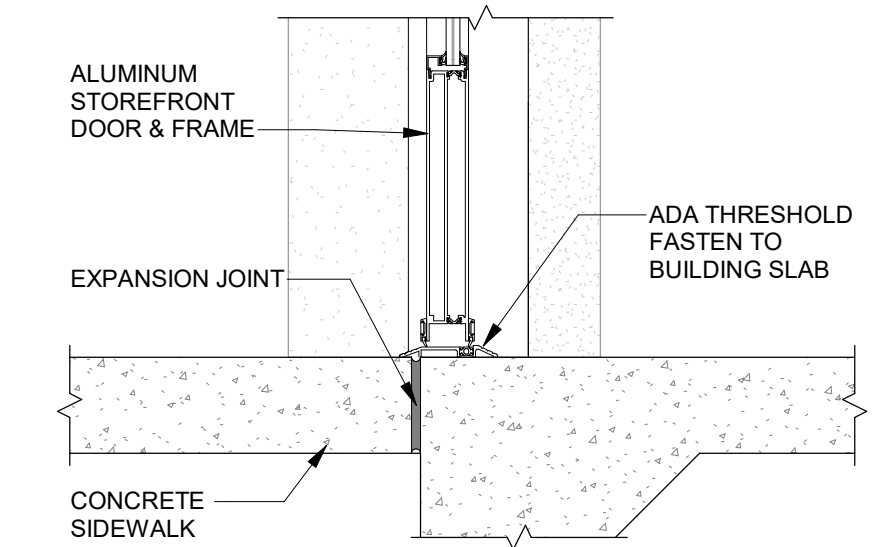
MARK	DESCRIPTION
X1	EXTERIOR THRESHOLD (ENSURE THRESHOLD MEETS ADA REQUIREMENTS - NOT EXCEED 1/2" TO THE EXTERIOR EXISTING GRADE)

PLAN REVIEW DATA

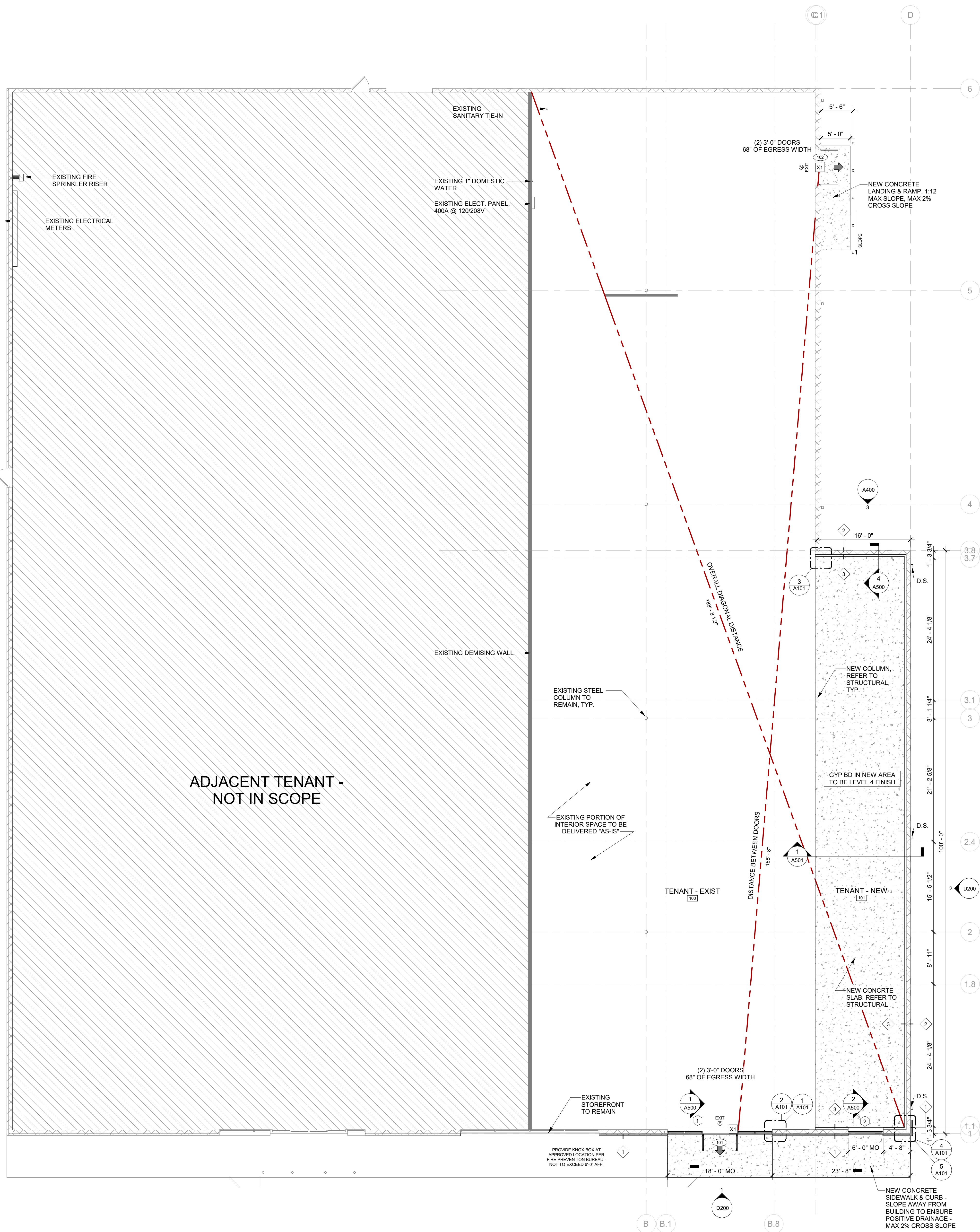
- I. APPLICABLE BUILDING CODES:**
 CITY OF McMINNVILLE, TN
 ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES CODE, 2017
 INTERNATIONAL BUILDING CODE, 2018
 INTERNATIONAL ENERGY CONSERVATION CODE, 2018
 INTERNATIONAL EXISTING BUILDING CODE, 2018
 INTERNATIONAL FIRE CODE, 2018
 INTERNATIONAL MECHANICAL CODE, 2018
 INTERNATIONAL PLUMBING CODE, 2018
 INTERNATIONAL PROPERTY MAINTENANCE CODE, 2018
 NATIONAL ELECTRIC CODE (NFPA 70), 2017
- II. OCCUPANCY GROUP:**
 A. PROPOSED: (M) MERCANTILE
 B. PREVIOUS: (M) MERCANTILE
- III. CONSTRUCTION TYPE:**
 A. I-B. UNPROTECTED, SPRINKLERED. ASSUMED CONSTRUCTION TYPE BASED ON OBSERVABLE CONDITIONS. EXTERIOR CMU WALLS, STEEL COLUMNS, BEAMS & BAY JOISTS, REINFORCED CONCRETE FLOOR CONSTRUCTION, METAL ROOF DECK, INTERIOR METAL STUD FRAMED PARTITIONS.
 **ALL NEW CONSTRUCTION TO COMPLY WITH TYPE I-B CONSTRUCTION.
- B. FIRE RESISTANCE REQUIREMENTS (TABLE 601)**
 1. STRUCTURAL FRAME: REQUIRED = 0 HOUR, PROVIDED = 0 HOUR
 2. EXT BEARING WALLS: REQUIRED = 0 HOUR, PROVIDED = 0 HOUR
 3. INT BEARING WALLS: REQUIRED = 0 HOUR, PROVIDED = 0 HOUR
 4. EXT NON-BEARING WALLS: REQUIRED = 0 HOUR, PROVIDED = 0 HOUR
 5. INT NON-BEARING WALLS: REQUIRED = 0 HOUR, PROVIDED = 0 HOUR
 6. FLOOR CONSTRUCTION: REQUIRED = 0 HOUR, PROVIDED = 0 HOUR
 7. ROOF CONSTRUCTION: REQUIRED = 0 HOUR, PROVIDED = 0 HOUR
- FIRE RESISTANCE RATING REQUIREMENTS FOR EXTERIOR WALLS BASED ON FIRE SEPARATION DISTANCE (TABLE 602):**
 LESS THAN 1' - 1 HOUR
 GREATER THAN OR EQUAL TO 5' LESS THAN 10' - 1 HOUR
 GREATER OR EQUAL TO 10' - NOT REQUIRED
- *ALL EXISTING EXTERIOR WALLS HAVE FIRE SEPARATION DISTANCE GREATER THAN 10'**
- IV. FIRE SUPPRESSION SYSTEM:**
 A. FULLY SPRINKLERED (903.2.1)
 B. EXISTING FIRE ALARM SYSTEM, MONITORING AND NOTIFICATION
 C. SPRINKLER AND FIRE ALARM SYSTEM MODIFICATIONS ARE DEFERRED SUBMITTAL ITEMS BY CONTRACTOR. CONTRACTOR SHALL SUBMIT SIGNED AND SEALED ENGINEERED DRAWINGS AND CALCULATIONS FOR MODIFICATIONS TO SPRINKLER AND ALARM SYSTEMS TO ARCHITECT/ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO SUBMITTAL TO REVIEWING AUTHORITY.
- V. BUILDING HEIGHT AND AREA: (CHAPTER 5 TABLES 504.3, 504.4, 506.2)**
 A. ALLOWABLE HEIGHT (M): 3 STORIES, 55'
 B. ACTUAL HEIGHT: 1 STORIES, 16'-10"
 C. ALLOWABLE AREA: 50,000 SF PER FLOOR
 D. ACTUAL PROJECT SQUARE FOOTAGE: 10,450 SF GROSS
 E. TOTAL BUILDING SQUARE FOOTAGE: 26,170 SF
- VI. OCCUPANT LOAD / EXIT CAPACITY: (SECTION 1004 - OCCUPANT LOAD (TABLE 1004.1.2))**
 MERCANTILE: 60 SF GROSS/OCC. (10,450 SF) = 175
 10,450 SF TOTAL / 175 TOTAL OCCUPANTS
- VII. NUMBER OF EXITS:**
 A. MINIMUM NUMBER OF EXITS (SECTION 1006):
 1. TOTAL OCCUPANT LOAD = 175 OCCUPANTS = 2 EXITS, MIN.
 B. NUMBER OF EXITS PROVIDED: 2
 C. EACH EXIT MUST ACCOMMODATE GREATER THAN 50% OF THE TOTAL OCCUPANT LOAD SO THAT LOSS OF ANY ONE EXIT DOES NOT REDUCE AVAILABLE CAPACITY TO LESS THAN 50% OF OCCUPANTS. 88 OCCUPANT CAPACITY REQUIRED EACH EXIT
- EXIT CAPACITY - CALCULATIONS (1005.3.2)**
 DOORS: 0.20' PER PERSON
 STAIRS: 0.30' PER PERSON
 EXIT 1: 68" CLEAR WIDTH AT DOOR - 340 OCCUPANT CAPACITY (OK)
 EXIT 2: 68" CLEAR WIDTH AT DOOR - 340 OCCUPANT CAPACITY (OK)
 TOTAL CAPACITY: 680 OCCUPANTS (OK)
- VIII. ARRANGEMENT OF EXITS:**
 A. SEPARATION OF EXITS: AT LEAST 2 EXITS SHALL BE PLACED A DISTANCE APART EQUAL TO NOT LESS THAN ONE-THIRD THE LENGTH OF THE MAXIMUM OVERALL DIAGONAL DIMENSION OF THE AREA BEING SERVED (1007.1.1) FOR SPRINKLERED BUILDINGS.
 B. MAXIMUM TRAVEL DISTANCE: 250' FULLY SPRINKLERED (TABLE 1017.2)
- IX. OTHER LIFE SAFETY CONSIDERATIONS (TENANT):**
 A. MINIMUM CORRIDOR WIDTH: 44" (1020.2)
 B. MINIMUM AISLE WIDTH: 42" WHERE BOTH SIDES SERVED; 36" WHERE ONE SIDE SERVED (1029.9.1)
 C. DEAD END CORRIDOR: 20' MAXIMUM (1020.4)
 D. DOOR CLEAR WIDTH: 32" MINIMUM (1010.1.1)
 E. NO EGRESS THRU STOCKROOMS
 F. INTERIOR FINISHES (TABLE 803.13) SPRINKLERED
 1. VERTICAL EXIT PASSAGES: CLASS B
 2. ROOMS / ENCLOSED SPACES: CLASS C
 3. FLOOR FINISHES: FLOOR FINISH MATERIALS MUST COMPLY WITH ASTM STANDARD E648, AND HAVING A SPECIFIC OPTICAL DENSITY SMOKE RATING NOT TO EXCEED 450 PER ASTM E822.



2 HM DOOR SILL - EXTERIOR2
 SCALE: 1 1/2" = 1'-0"



3 STOREFRONT DOOR SILL
 SCALE: 1 1/2" = 1'-0"



1 FLOOR PLAN
 SCALE: 1/8" = 1'-0"

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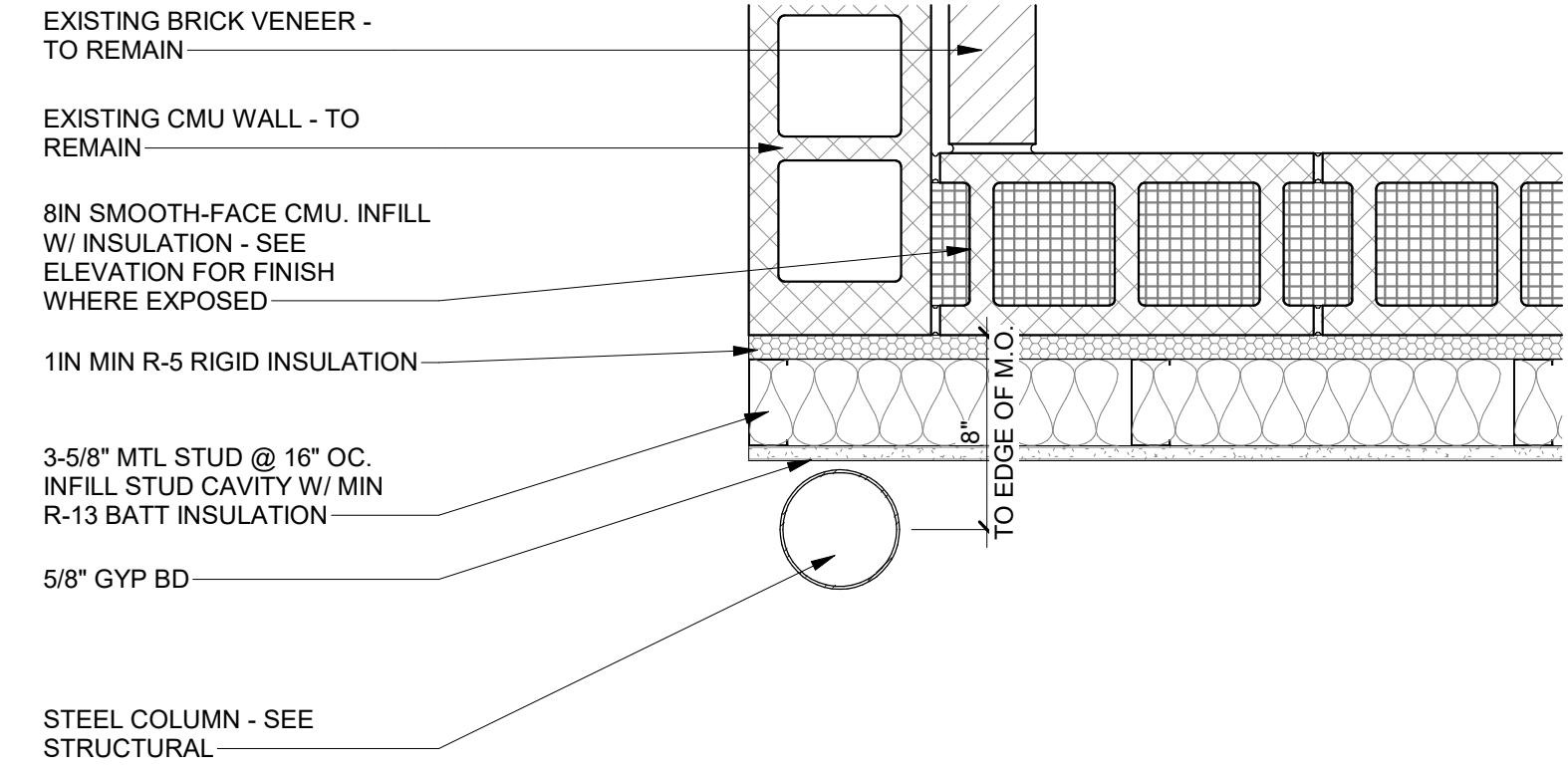
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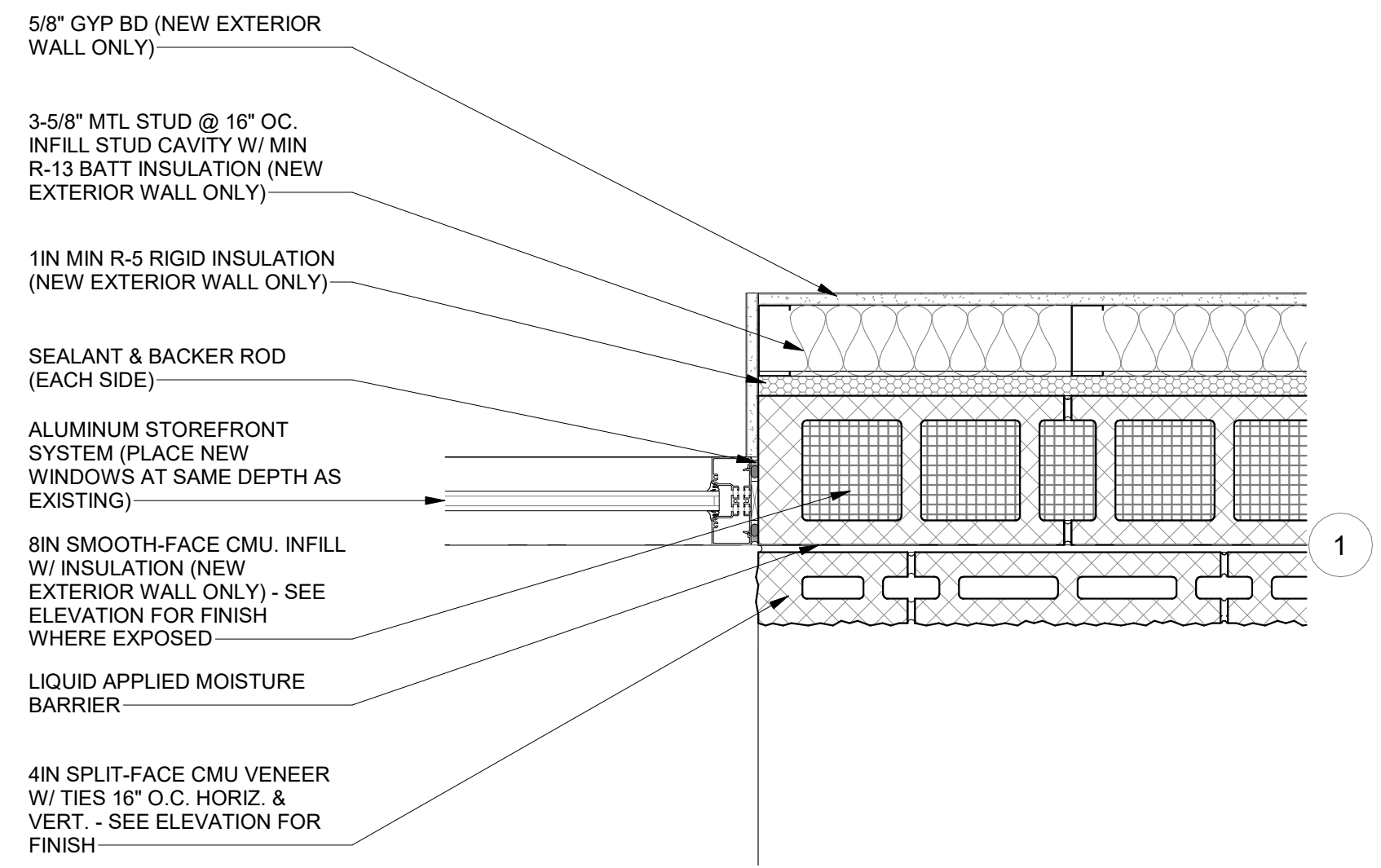
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PLAN DETAILS

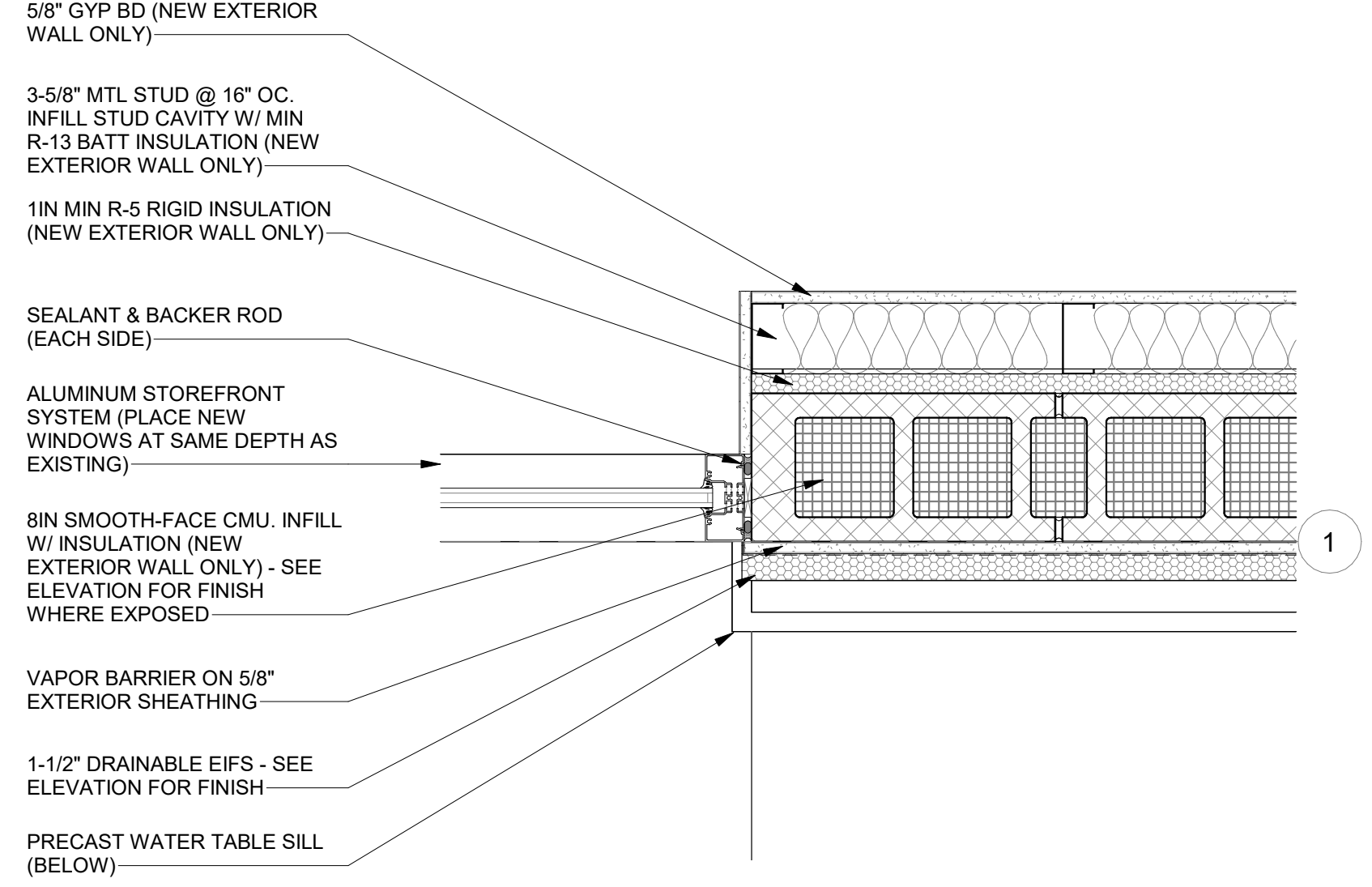
A101



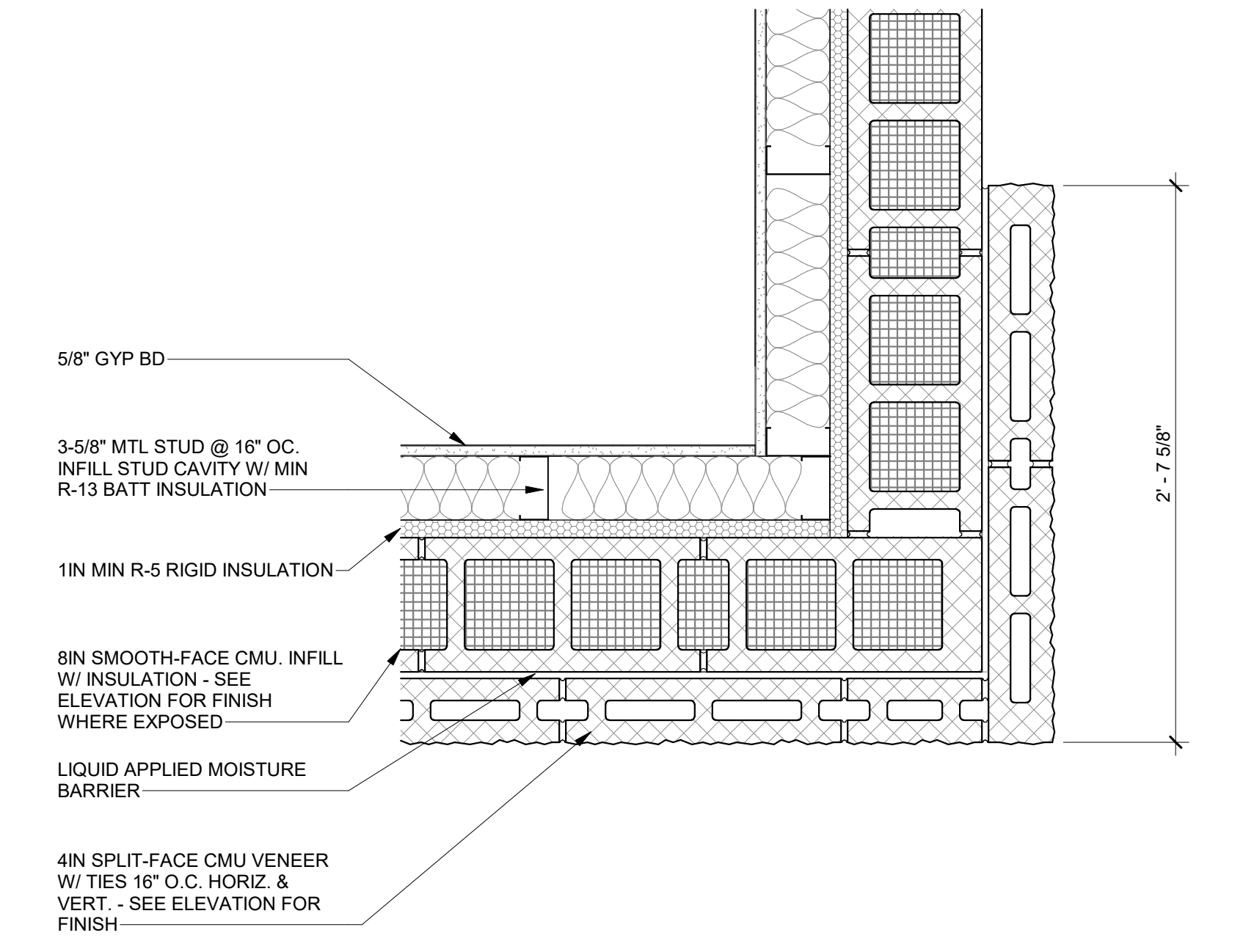
3 DETAIL @ NEW/EXIST WALL
SCALE: 1 1/2" = 1'-0"



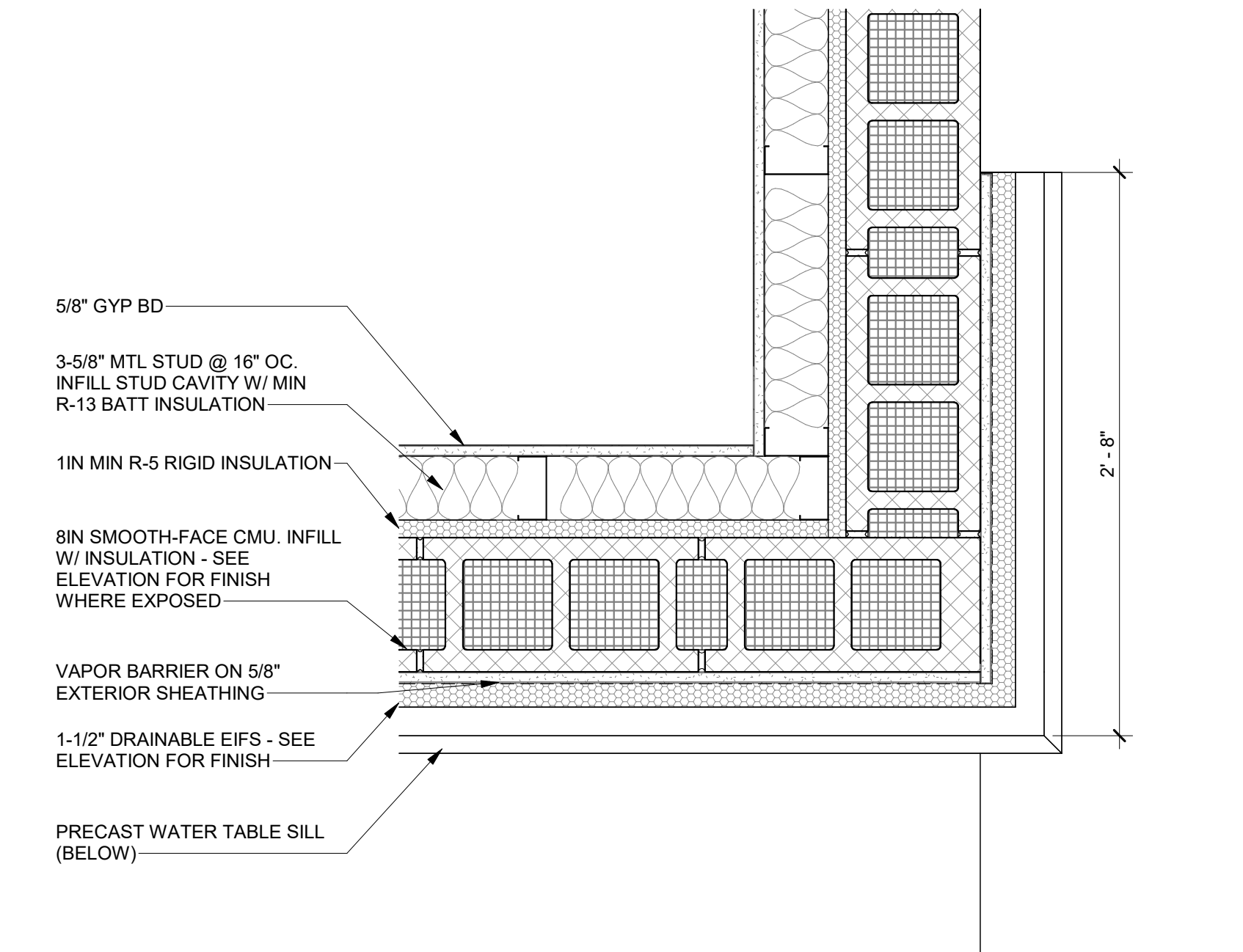
2 DETAIL @ SF JAMB - CMU
SCALE: 1 1/2" = 1'-0"



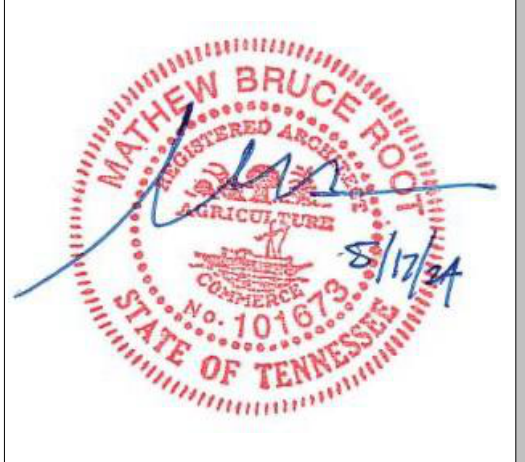
1 DETAIL @ SF JAMB - EIFS
SCALE: 1 1/2" = 1'-0"



5 DETAIL @ CORNER - CMU
SCALE: 1 1/2" = 1'-0"



4 DETAIL @ CORNER - EIFS
SCALE: 1 1/2" = 1'-0"



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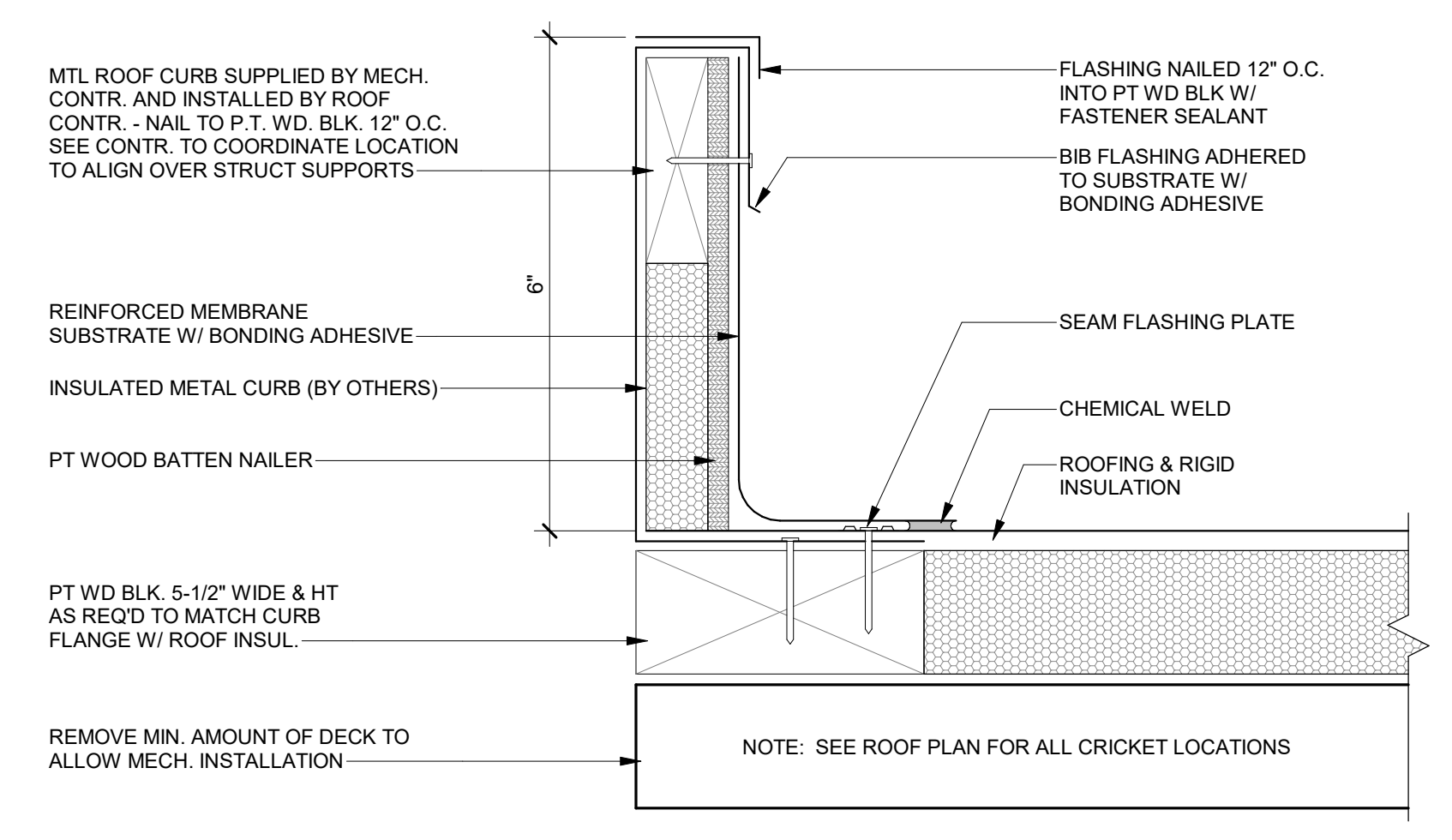
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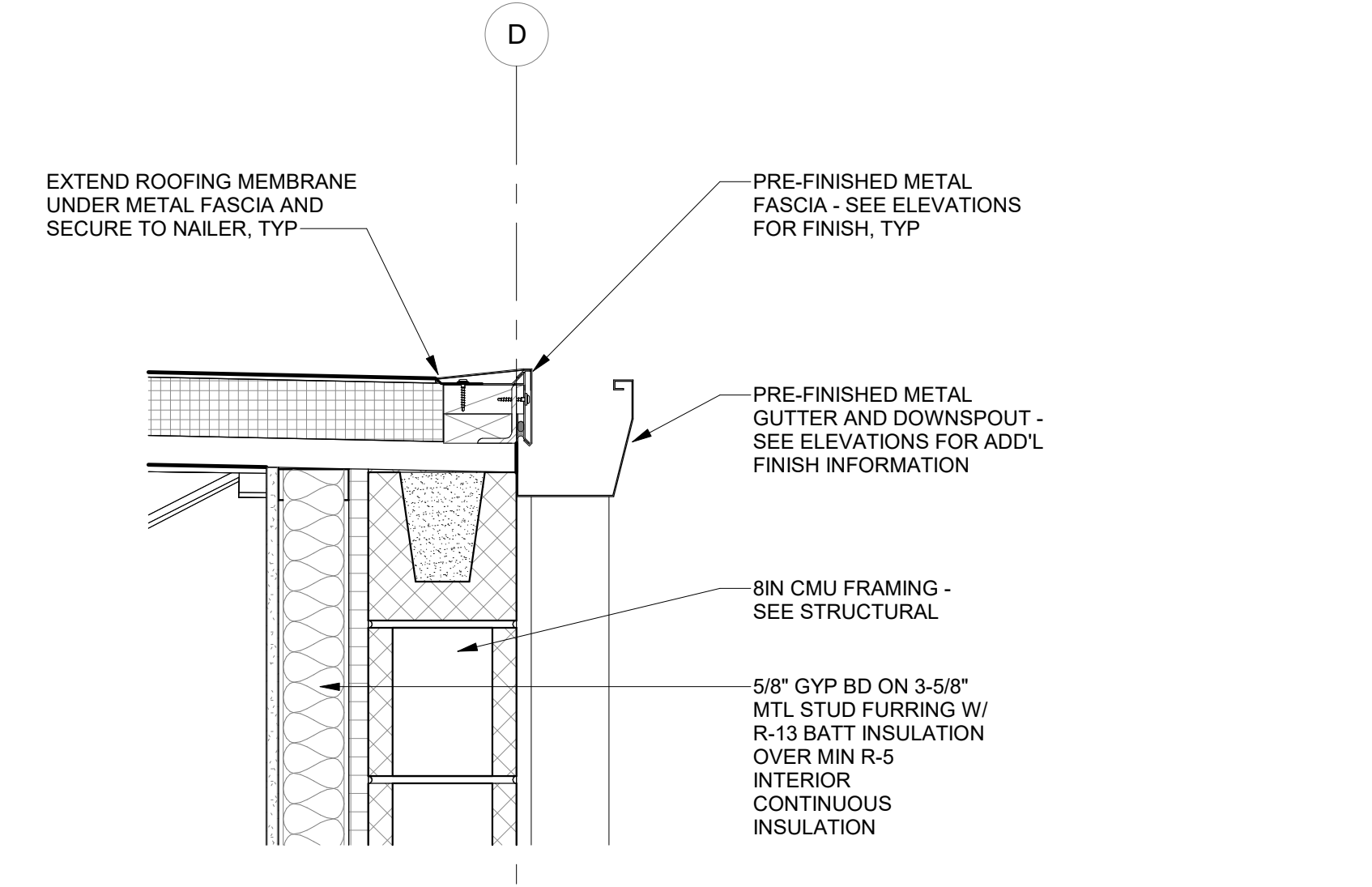
REVISIONS
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A201

ROOF NOTES

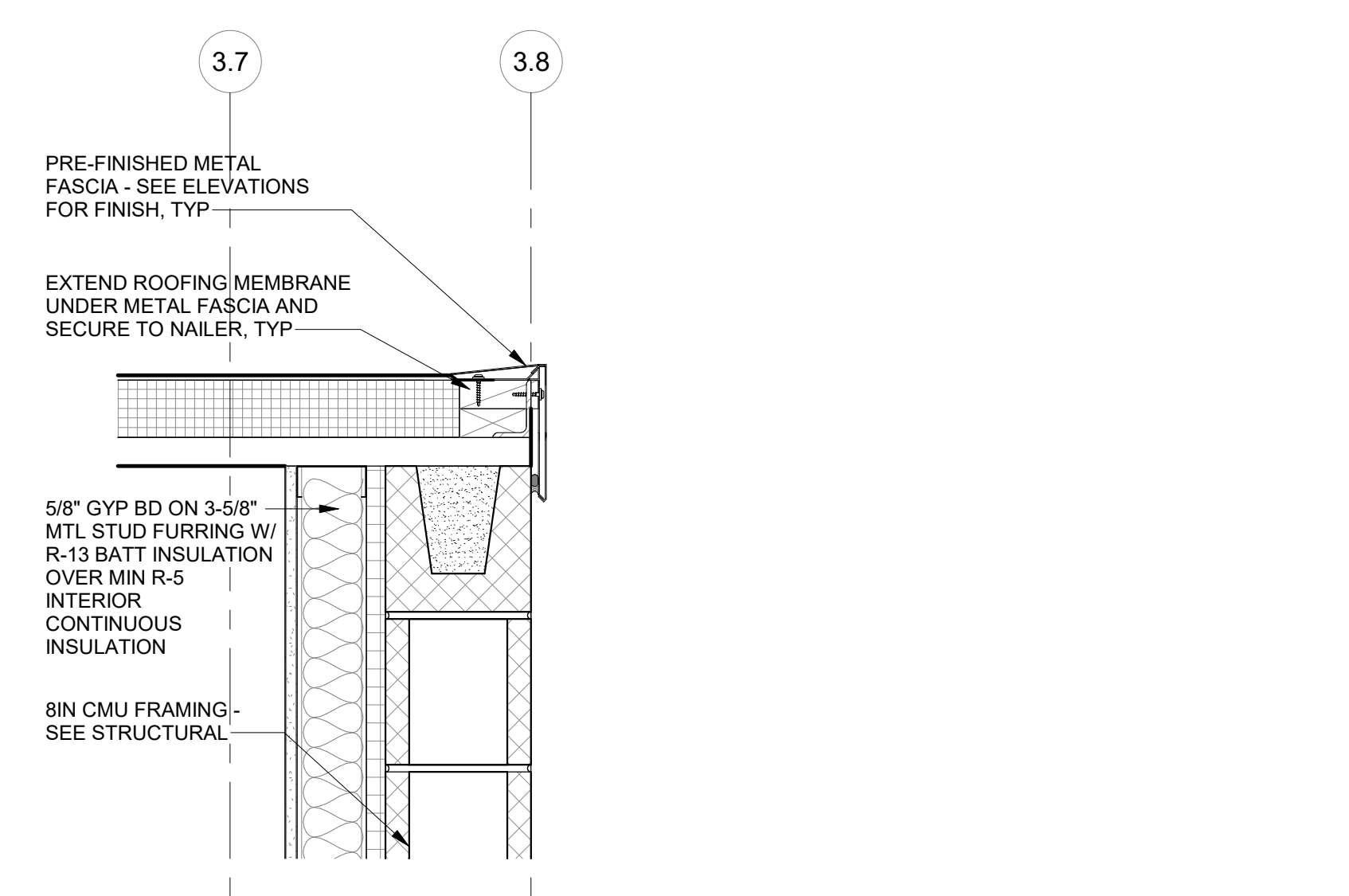
1. ALL NEW PENETRATIONS TO BE FLASHED TO EXISTING ROOF W/ COMPATIBLE MATERIALS. LANDLORD ROOFING CONTRACTOR TO BE USED FOR ALL WORK ON EXISTING ROOF.
2. CONTRACTOR TO SCHEDULE WALKTHROUGH W/ LANDLORD ROOFER AND PREPARE LIST OF ITEMS NEEDING REPAIR (FLASHING, ROOFING, DRAINS, COPING, ETC.) FOR REVIEW BY LANDLORD.



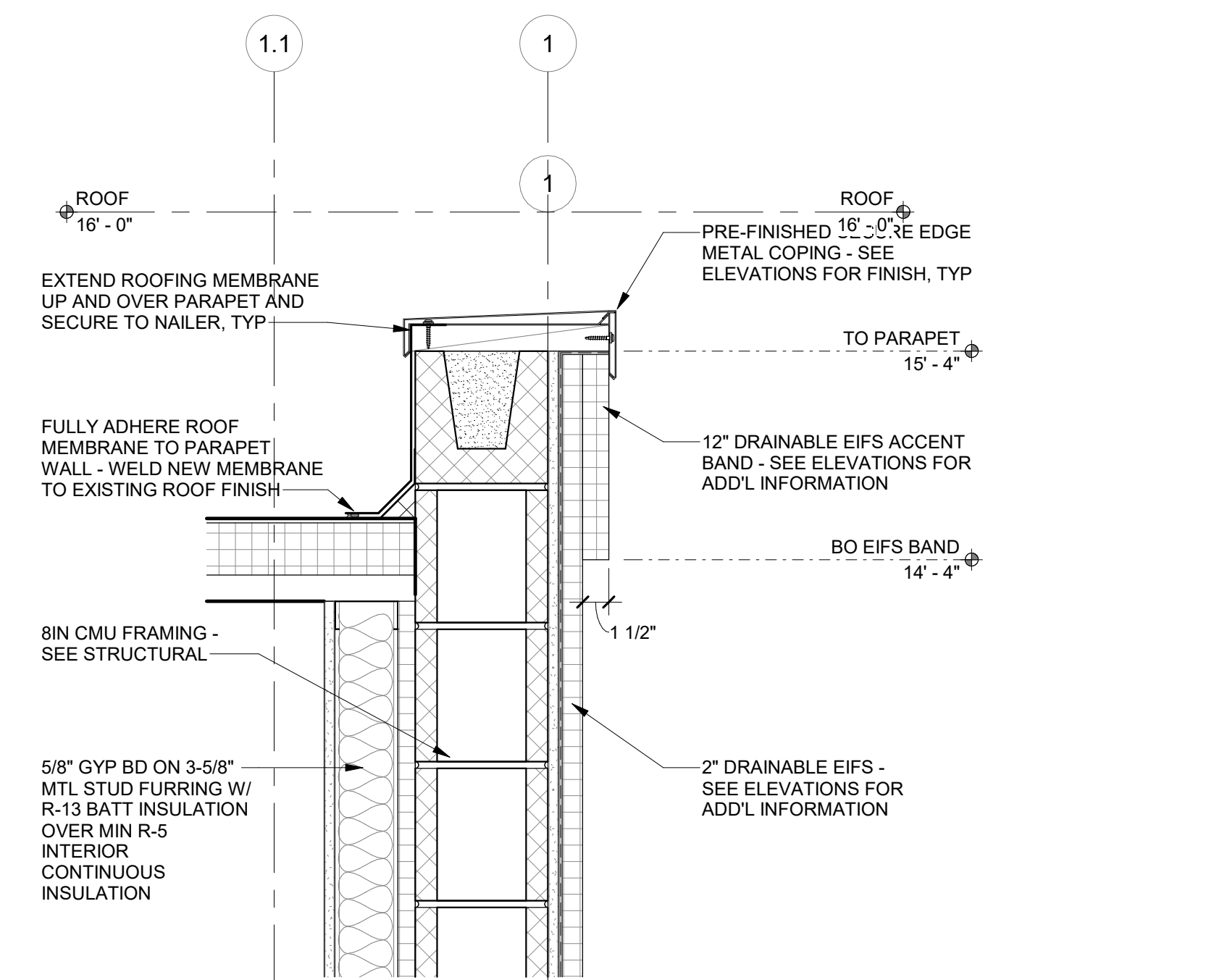
2 HVAC CURB DETAIL
 SCALE: 1/2" = 1'-0"



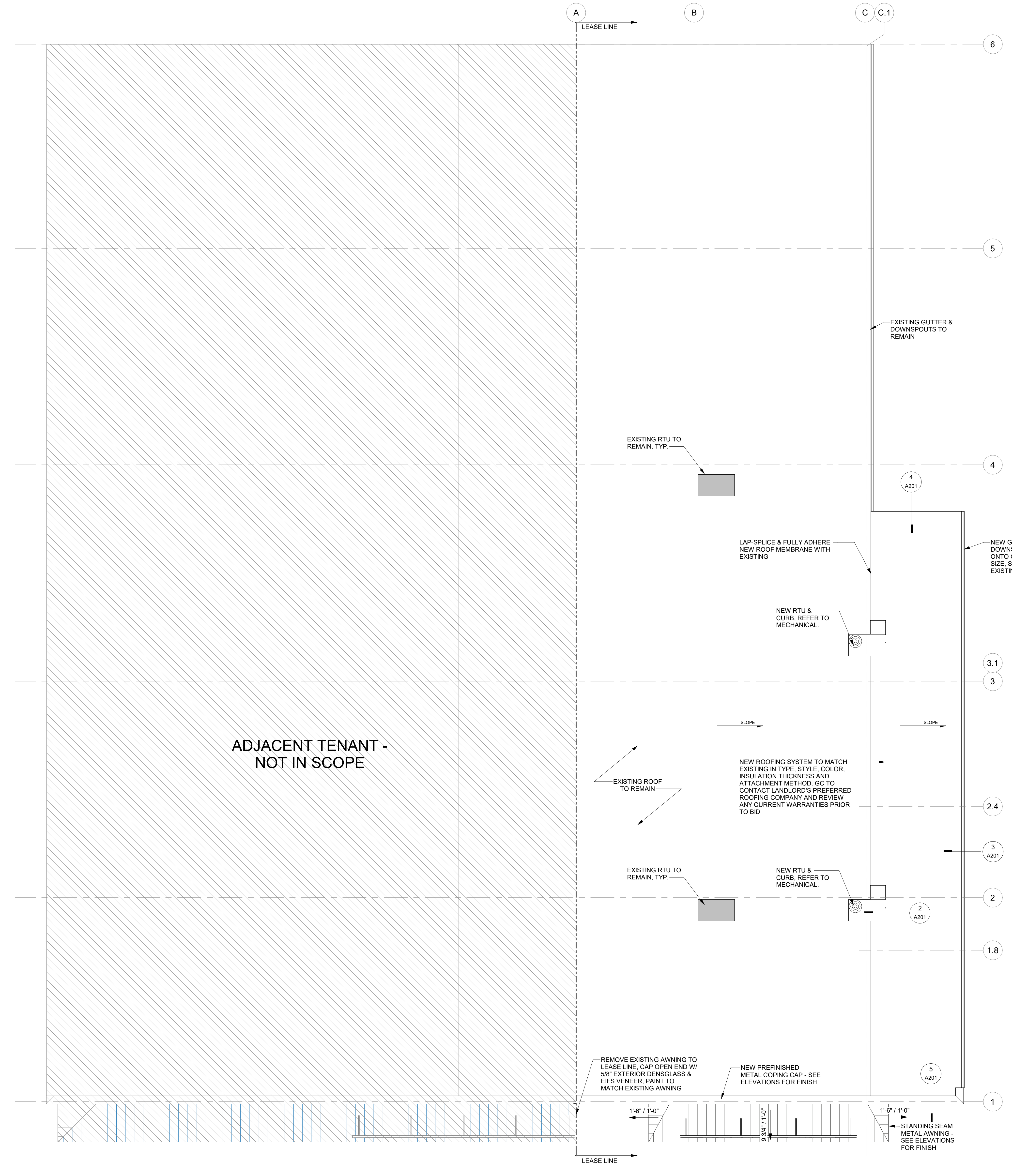
3 DETAIL @ GUTTER & DOWNSPOUT
 SCALE: 1 1/2" = 1'-0"



4 DETAIL @ PARAPET - CMU
 SCALE: 1 1/2" = 1'-0"



5 DETAIL @ PARAPET - EIFS
 SCALE: 1 1/2" = 1'-0"



1 ROOF PLAN
 SCALE: 1/8" = 1'-0"

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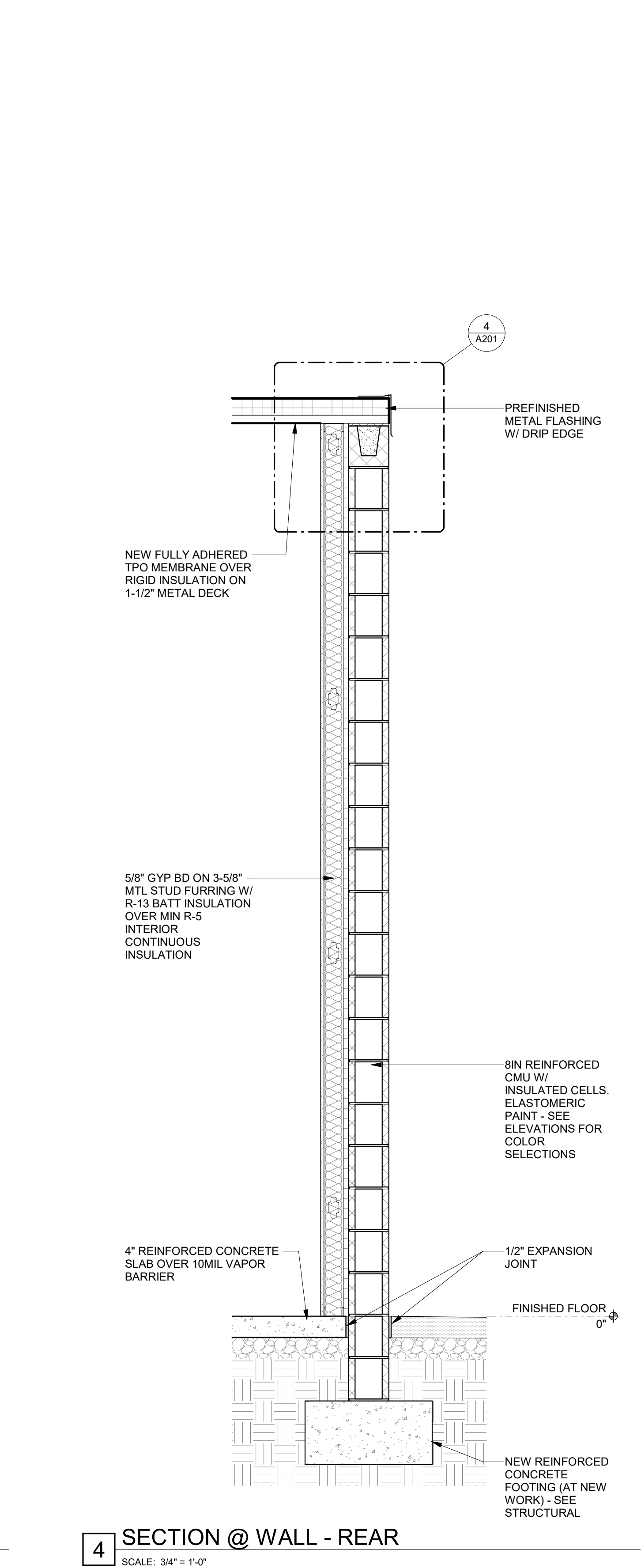
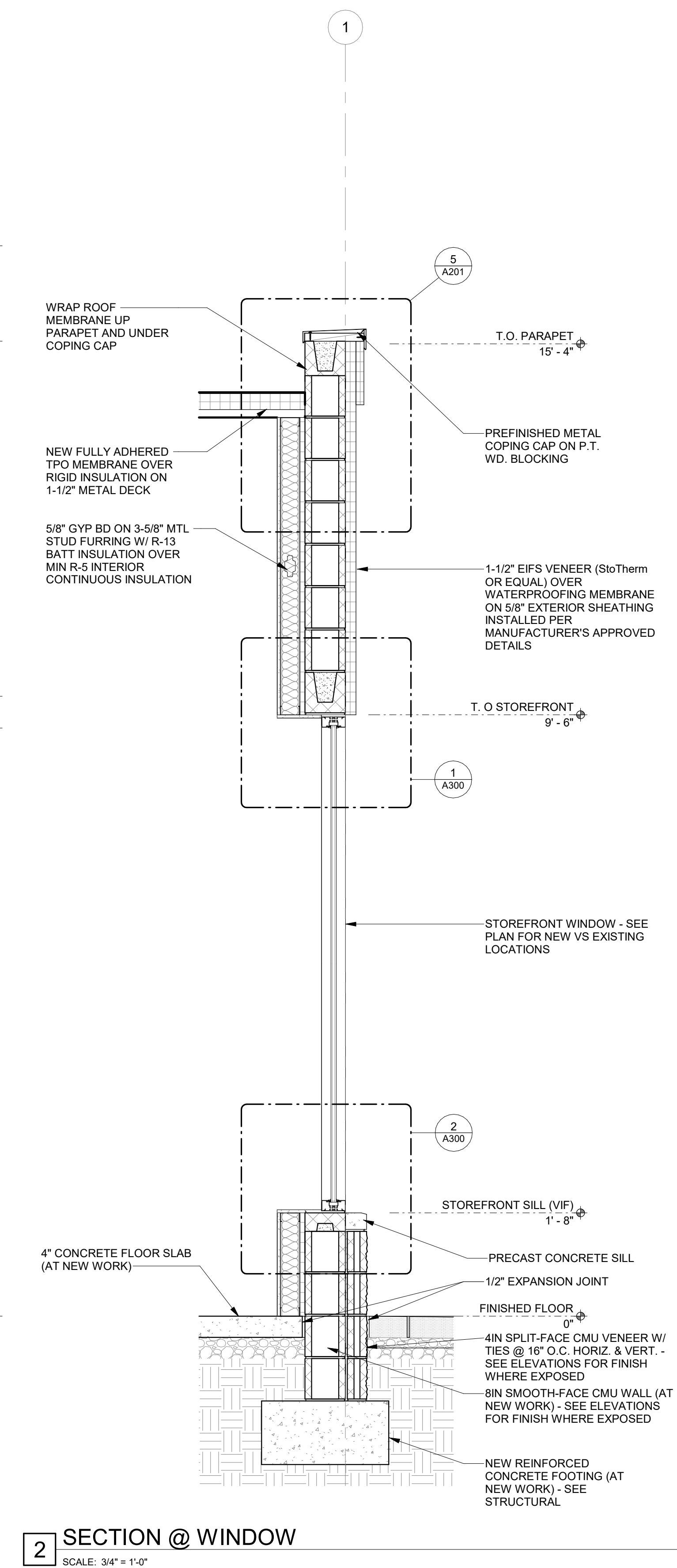
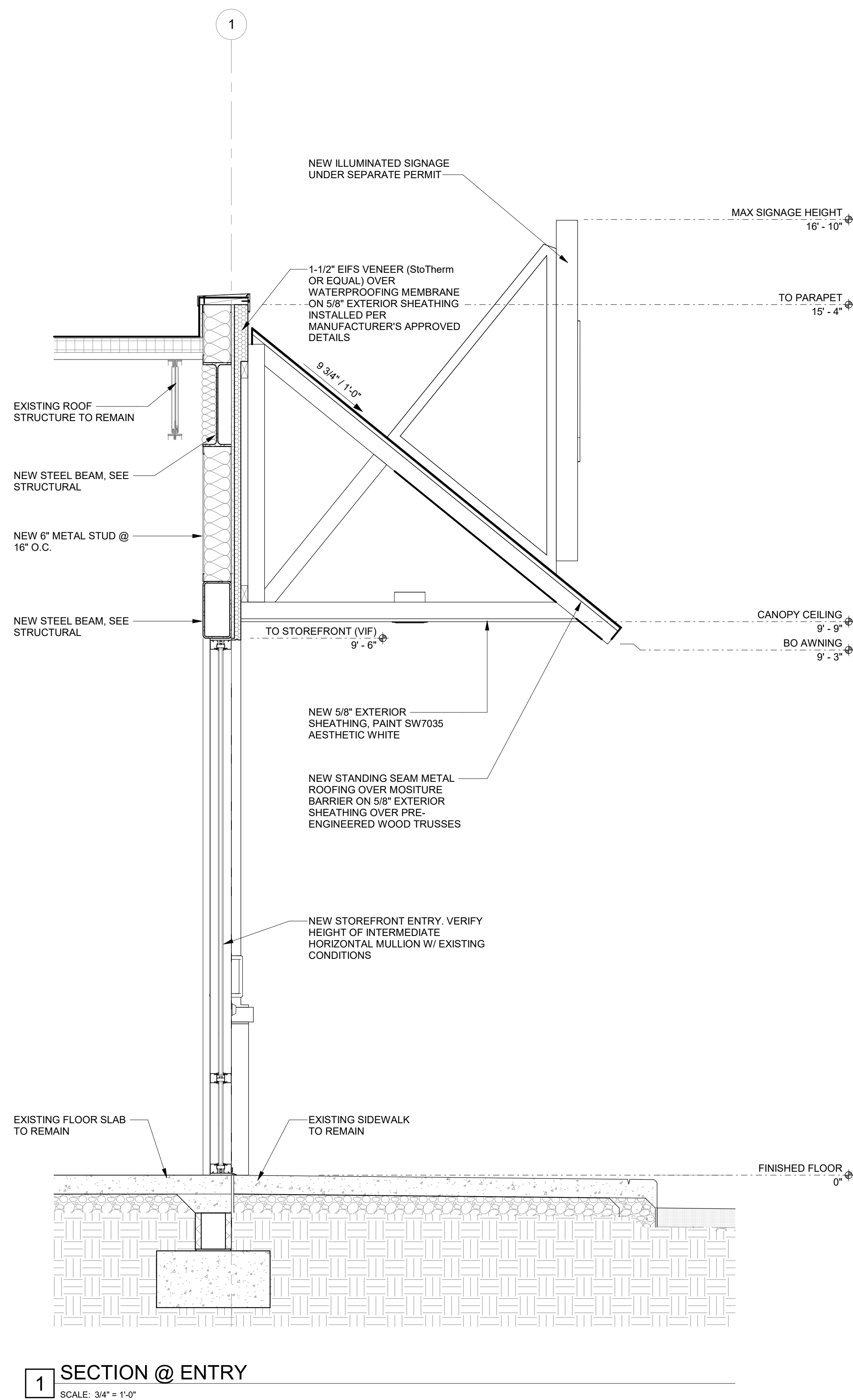
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WALL SECTIONS

A500





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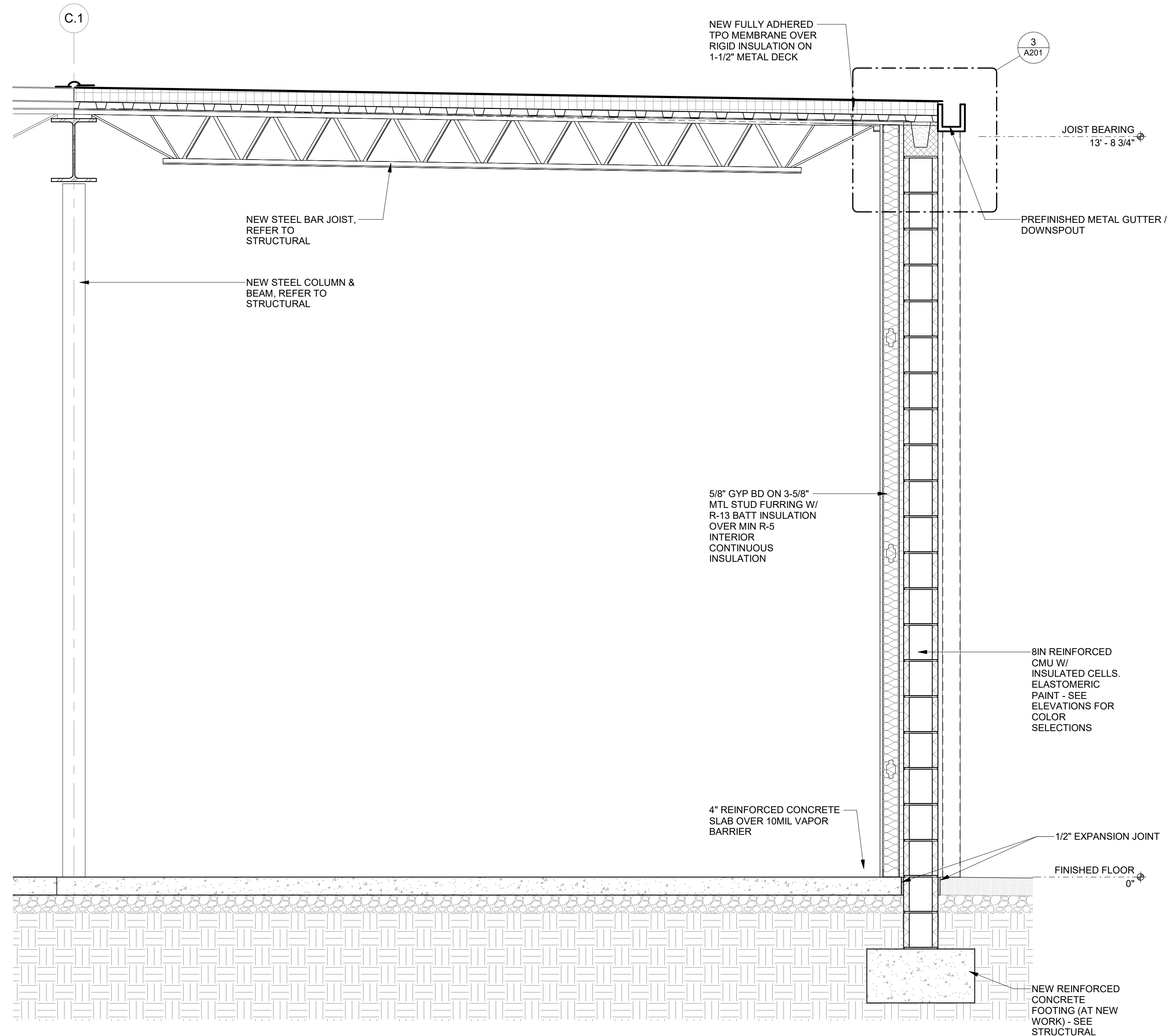
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WALL SECTIONS

A501

SHEET



1 SECTION @ WALL - SIDE
 SCALE: 3/4" = 1'-0"

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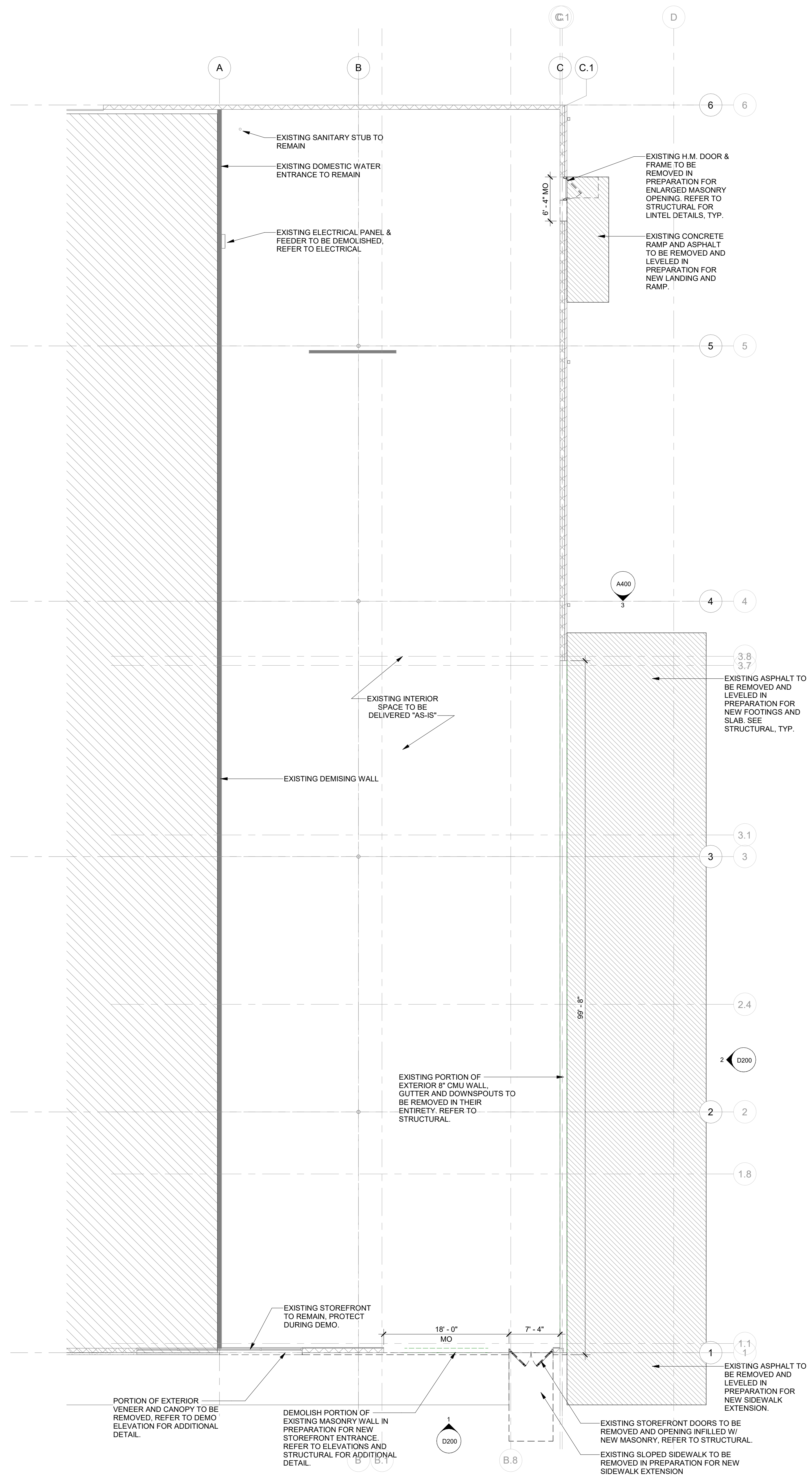
LOCATION

REVISIONS

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DEMOLITION NOTES

- GC TO FIELD VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS PRIOR TO CONSTRUCTION. ANY CONFLICTS, OMISSIONS, ETC. SHALL BE IMMEDIATELY REPORTED TO THE ARCHITECT FOR CLARIFICATION PRIOR TO PERFORMANCE OF ANY WORK IN QUESTION.
- GC IS RESPONSIBLE FOR PHASING OF DEMO WITH NEW WORK OF TENANT PACKAGE. SUB-CONTRACTORS TO VISIT THE SITE AND BECOME FAMILIAR WITH THE SCOPE OF WORK PRIOR TO STARTING ANY DEMOLITION.
- PREVENT MOVEMENT OR SETTLEMENT OF STRUCTURE(S). CONTRACTOR SHALL PROVIDE AND PLACE BRACING OR SHORING AND BE RESPONSIBLE FOR SAFETY AND SUPPORT OF STRUCTURE AND SHALL ASSUME LIABILITY FOR SUCH MOVEMENT, SETTLEMENT, DAMAGE, OR INJURY.
- CONTRACTOR SHALL CEASE OPERATIONS AND NOTIFY THE OWNER & ARCHITECT IMMEDIATELY IF SAFETY OF STRUCTURE APPEARS TO BE ENDANGERED. TAKE PRECAUTIONS TO PROPERLY SUPPORT STRUCTURE. DO NOT RESUME OPERATIONS UNTIL SAFETY IS RESTORED.
- PROVIDE, ERECT AND MAINTAIN BARRICADES, LIGHTING AND GUARD RAILS AS REQUIRED BY APPLICABLE REGULATORY AGENCIES TO PROTECT OCCUPANTS OF THE BUILDING, WORKERS, ADJACENT TENANTS AND THE PUBLIC.
- CONTRACTOR SHALL ARRANGE AND PAY FOR DISCONNECTING, REMOVING, AND CAPPING UTILITY SERVICES WITHIN THE AREA OF DEMOLITION AS WELL AS IDENTIFYING SERVICE LINES AND CAPPING LOCATIONS ON PROJECT RECORD DOCUMENTS. CONTRACTOR SHALL PLACE MARKERS TO INDICATE LOCATION OF DISCONNECTED SERVICES, AND COORDINATE ANY AND ALL INTERRUPTIONS OF UTILITIES WITH CURRENT TENANTS. 48 HOURS WRITTEN NOTICE OF ANY INTENDED INTERRUPTION MUST BE PRESENTED TO THE OWNER.
- DEMOLISH IN AN ORDERLY AND CAREFUL MANNER AS REQUIRED TO ACCOMMODATE FUTURE TENANT IMPROVEMENTS. PERFORM DEMOLITION IN ACCORDANCE WITH APPLICABLE AUTHORITIES HAVING JURISDICTION.
- REPAIR ALL DEMOLITION PERFORMED IN EXCESS OF THAT REQUIRED, AT NO ADDITIONAL COST TO THE OWNER.
- BURNING OF MATERIALS ON SITE IS NOT PERMITTED. REMOVE DEMOLISHED MATERIALS, TOOLS AND EQUIPMENT FROM SITE UPON COMPLETION OF WORK. SITE SHALL BE CLEANED DAILY. COORDINATE DEMO WORK WITH AUTHORITIES HAVING JURISDICTION AND LANDLORD FOR ALLOWABLE HOURS OF OPERATION.
- DEMOLITION NOTES PERTAIN TO THE GENERAL SCOPE OF WORK AND ARE TO BE CONSIDERED AS PERTAINING TO ALL SIMILAR CONDITIONS WHETHER SPECIFICALLY INDICATED OR NOT. IF SCOPE IS IN QUESTION, CONTACT OWNER'S REPRESENTATIVE IMMEDIATELY.
- ALL ORIGINAL CODE REQUIRED FIRE RATED PARTITIONS, FIREPROOFING, FIRE-RATED ASSEMBLIES, ETC. DAMAGED DURING CONSTRUCTION OR IN OTHERWISE DISREPAIR SHALL BE REPAIRED AND RETURNED TO CURRENT CODE REQUIRED RATINGS.
- CONTRACTOR SHALL PROVIDE TEMPORARY ENCLOSURES, AS REQUIRED TO SECURE BUILDING AND PROTECT FROM THE ELEMENTS. CONTRACTOR SHALL COORDINATE SEQUENCING OF INTERIOR & EXTERIOR DEMOLITION WITH FUTURE WORK TO MINIMIZE BUILDING EXPOSURE TO THE ELEMENTS.
- WALLS, PARTITIONS, DOORS, FRAMES, AND OTHER ITEMS TO BE REMOVED ARE SHOWN DASHED. SERVICES WITHIN WALLS AND PARTITIONS SHALL ALSO BE REMOVED. EDGES OF WALLS SHOWN TO REMAIN SHALL BE SAWCUT OR CLEANLY TOOTHED TO ACCEPT NEW CONSTRUCTION. REPAIR AND PATCH EXISTING WALLS SHOWN TO REMAIN WHERE INTERSECTING WALLS, DOORS, FRAMES, ETC. ARE SHOWN TO BE REMOVED AND WHERE EXISTING CONSTRUCTION WILL NOW BE EXPOSED IN THE NEW CONSTRUCTION. EXISTING CONSTRUCTION SHOWN TO REMAIN, INCLUDING BUT NOT LIMITED TO WALLS, PARTITIONS, DOORS, FRAMES, ETC. SHALL BE PROTECTED DURING DEMOLITION. DAMAGE TO EXISTING CONSTRUCTION SHOWN TO REMAIN SHALL BE RESTORED TO MATCH PRE-DAMAGED CONDITION. ALL INFILL OR REPLACEMENT WORK SHALL MATCH EXISTING CONDITIONS IN MATERIALS, CONSTRUCTION AND FINISH, UNLESS SPECIFICALLY NOTED ELSEWHERE IN THE CONSTRUCTION DOCUMENTS. PROTECT FROM DAMAGE ALL EXISTING FINISH WORK TO REMAIN IN PLACE AND WHICH BECOMES EXPOSED DURING DEMOLITION OPERATIONS.
- EXISTING CONCRETE FLOOR SLABS, MASONRY WALLS AND EXISTING STRUCTURAL FRAMING SYSTEMS SHOWN TO BE REMOVED SHALL BE CLEANLY SAWCUT FROM EXISTING CONSTRUCTION. ALSO COMPLETELY REMOVE FOOTINGS, FOUNDATIONS AND ABOVE GROUND AND UNDERGROUND CONSTRUCTION.
- REMOVE ALL EXISTING MISCELLANEOUS NON-LOAD BEARING ITEMS NOT BEING RE-USED, IN THEIR ENTIRETY, THROUGHOUT TENANT SPACE ABOVE AND BELOW EXISTING CEILING, INCLUDING (BUT NOT LIMITED TO) PLASTER AND DRYWALL, PARTITIONS, DOORS, FRAMES, SOFFITS, STUDS, FURRING, INSULATION, CEILING SUSPENSION SYSTEMS, ETC. PARTICULARLY WHERE EXISTING ITEMS WILL INTERFERE WITH THE INSTALLATION OF NEW CONSTRUCTION, OR WHERE EXISTING ITEMS WILL BE EXPOSED IN THE NEW CONSTRUCTION, UNLESS SPECIFICALLY SHOWN ELSEWHERE IN THE CONTRACT DOCUMENTS TO REMAIN. REPAIR AND PATCH ALL SURFACES TO REMAIN, WITH MATERIALS MATCHING EXISTING CONSTRUCTION. COORDINATE WITH NEW CONSTRUCTION.
- WHERE FINISHES ARE SHOWN TO BE REMOVED FROM EXISTING CONSTRUCTION, REPAIR AND PATCH REMAINING SUBSTRATE AND PREPARE FOR NEW FINISH AS PROVIDED IN SCHEDULES OR NOTED ON DRAWINGS.
- REMOVE ALL EXISTING PLUMBING, MECHANICAL AND ELECTRICAL EQUIPMENT NOT BEING RE-USED, IN THEIR ENTIRETY, THROUGHOUT TENANT SPACE PARTICULARLY WHERE EXISTING ITEMS WILL INTERFERE WITH THE INSTALLATION OF NEW CONSTRUCTION, OR WHERE EXISTING ITEMS WILL BE EXPOSED IN THE NEW CONSTRUCTION, UNLESS SPECIFICALLY SHOWN ELSEWHERE IN THE CONTRACT DOCUMENTS TO REMAIN, WITH MATERIALS MATCHING EXISTING CONSTRUCTION. COORDINATE WITH NEW CONSTRUCTION.
- ALL EXISTING SPRINKLER HEADS, DROPS AND BRANCH LINES AT LAY-IN CEILING AREA TO BE REMOVED IN ENTIRETY AND REPLACED WITH UPRIGHT HEAD SYSTEM IN OPEN STRUCTURE. ALL MANS AND BRANCHES THAT ARE SUITABLE FOR REUSE AND LOCATED WITHIN 18" OF BOTTOM OF STRUCTURE MAY BE REUSED. ALL LINES LOCATED BELOW THAT LIMIT SHALL BE DEMOLISHED OR RAISED TO WITHIN 18" OF BOTTOM OF STRUCTURE. SPRINKLER SUBCONTRACTOR IS RESPONSIBLE FOR DESIGN AND PERMITTING OF SPRINKLER SYSTEM WITH AUTHORITY HAVING JURISDICTION.



1 FLOOR PLAN - DEMOLITION
SCALE: 1/8" = 1'-0"

FLETCHER BRIGHT
COMPANY

1007 ASHLAND TERRACE
SUITE 104A
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DOLLAR TREE
SHELL -
MCMINNVILLE

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DEMOLITION
PLAN

D100

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 SUITE 104A
 CHATTANOOGA, TN 37415

DOLLAR TREE SHELL -
 McMINNVILLE

LOCATION

912 N CHANCERY ST
 McMINNVILLE, TN 37110
 WARREN COUNTY

PERMIT SET
05/16/24
 MJM # 23336

DATE

NO.	DESCRIPTION	DATE

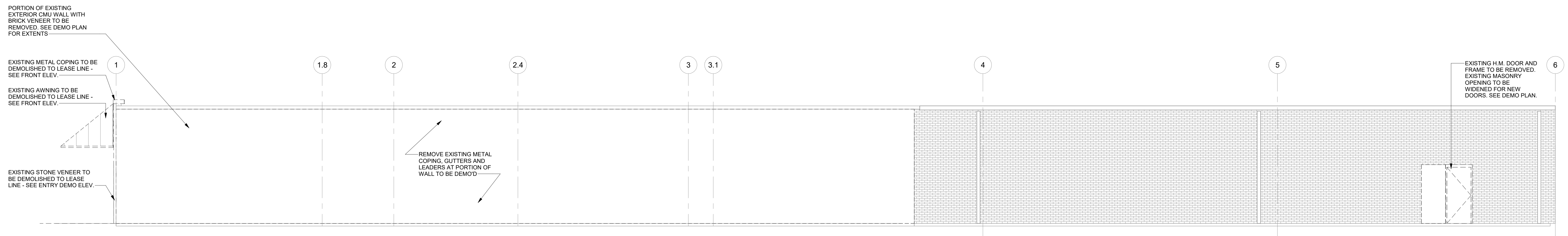
REVISIONS

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DEMOLITION ELEVATIONS

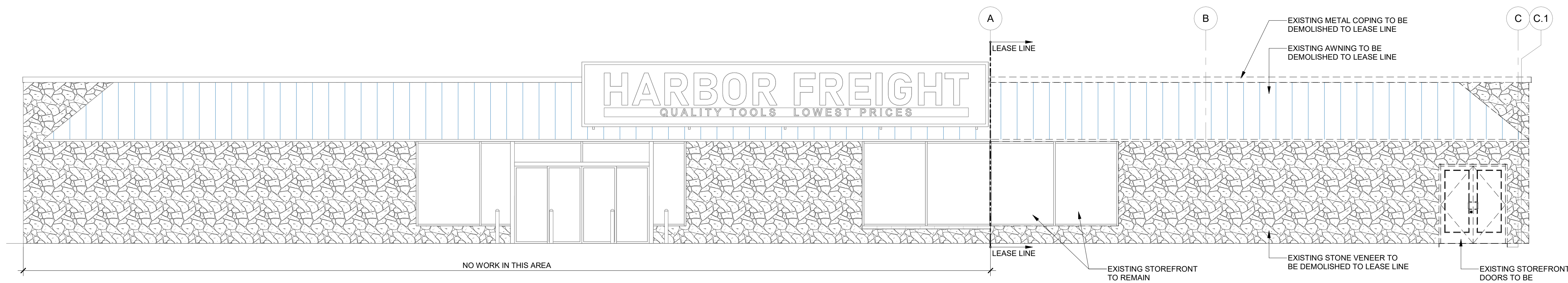
D200

SHEET



2 EXT ELEV @ SIDE - DEMO

SCALE: 3/16" = 1'-0"



1 EXT ELEV @ ENTRY - DEMO

SCALE: 3/16" = 1'-0"

STRUCTURAL GENERAL NOTES

1.0 DESIGN AND CODE INFORMATION

- 1.1 The Structure is designed based on the **2018 International Building Code**. All Design, Materials and Workmanship shall conform to the Referenced Code(s). Components designed by Suppliers shall be designed in accordance with the Referenced Code(s).
- 1.2 Superimposed Loading for Mechanical Units are based on the Weight of the Equipment as indicated on the Structural Drawings. Any change in Type, Location, Size or Number of Pieces of Equipment shall be reported to the Architect for verification of the adequacy of supporting members prior to the placement of equipment.
- 1.3 Roof Dead Loads: 20 psf (ASSUMED - FIELD VERIFY)
- 1.4 Structural Roof Members are designed for Minimum Roof Live Loads (Lr) based on the following Tributary loaded areas:
- Lr = 20 R₁ R₂ psf
Where: R₁ = 1 For 200' ≤ A ≤ 600 S.F.
R₁ = 1.2 - 0.001 A^{0.5} For 300' ≤ A ≤ 600 S.F.
R₁ = 0.6 For A ≤ 600 S.F.
And Where: R₂ = 1
- 1.5 Ground Snow Load = 10 psf
- 1.6 The Seismic loads are based on the following Earthquake Load Data:
- Seismic Importance Factor: 1.0
- Risk Category: II
- Site Class: D (Assumed)
- Seismic Design Category: C
Mapped Spectral Response Accelerations:
S_a = .255
S₁ = .112
Design Spectral Response Coefficients:
S_{DS} = .271
S_{1S} = .177
- 1.7 The Structure and its Components are designed for Wind Loads as defined by the Building Code, where:
- Basic Wind Speed (3 second gust): 105 mph (Ultimate)
- Wind Importance Factor: I = 1.00
- Risk Category: II
- Wind Exposure: C
- Internal Pressure Coefficient: +0.18/-0.18

2.0 ADDITIONAL CONTRACTOR RESPONSIBILITIES AND DEFINITIONS

- 2.1 Shop drawings shall not be reviewed for approval unless checked by the fabricator and approved by the contractor as indicated by the inclusion of the contractor's submittal review stamp. Submit one high quality PDF for review. Only marked up PDF will be returned. Reproduction of contract documents for shop drawings will not be permitted or accepted. See specifications and general notes for additional requirements.
- 2.2 The contractor shall provide **10 working days** in his schedule for the Structural Engineer's review of each submittal.
- 2.3 It is the Contractor's responsibility to certify that he has not made changes to specified materials on submittals.
- 2.4 The contractor shall insure that all construction loads do not exceed the design live loads indicated on the structural drawings and that these loads are not put on the structural members prior to the time that all framing members and their connections are in place.
- 2.5 Contractor shall verify and bear the expense of verifying all existing conditions, horizontal and vertical dimensions and coordination of Architectural and Structural drawings. Immediately notify the Architect of any discrepancies. For dimensions not shown on Structural Drawings, see the Architectural Drawings.
- 2.6 Dimensions shown indicate spans for which members are Structurally Adequate. The contractor shall COORDINATE AND CONFIRM ALL DIMENSIONS. Contractor shall include in his bid the Costs of Dimensional Coordination and Confirmation. These Documents constitute a performance specification. Coordinate all openings, support systems, duct work locations, mechanical elements, sprinklers, etc., with Structural elements. Consult the Engineer/Architect and obtain Approval PRIOR TO making changes to Structural Systems.
- 2.7 These Drawings and Specifications are performance specifications. Provide all labor, materials, equipment and services required to execute and complete all items of work as shown or indicated on the drawings and as specified in this section, including incidental items to effect a finished and complete job, even though such items are not shown or particularly mentioned on the Contract Documents.
- 2.8 The Contractor shall compare the Structural Sections with the Architectural Sections and report any discrepancies to the Architect PRIOR TO fabricating or installing Structural Members.
- 2.9 See Architectural Drawings or Specifications for any fireproofing requirements of structural members.
- 2.10 The design, adequacy and safety of erection bracing, shoring, temporary supports, etc., is the Sole Responsibility of the Contractor.
- 2.11 Erection and bracing of steel structures shall comply with the limits and recommendations of the "Code of Standard Practice", Latest edition of the American Institute of Steel Construction. Provide bracing wherever necessary to take care of all loads to which the structure may be subjected, including equipment and the operation of the same.
- 2.12 The details shown on the Structural Drawings designated as "Typical Details" apply generally to the drawings in all areas where conditions are similar to those described in the details.
- 2.13 Notes on the Structural General Notes sheet are applicable unless specifically noted otherwise on the drawings.
- 2.14 Principal openings are shown on the drawings. See Architectural, Mechanical and Electrical drawings for sleeves, curbs, inserts and anchors not shown. The contractor shall provide for all openings, whether shown on the Structural Drawings or not. Size and location of all openings shall be verified with the mechanical and electrical contractors. Any deviation from openings shown on the Structural Drawings shall be brought to the Engineer's attention for Approval PRIOR TO fabrication or installation of Structural Members.
- 2.15 Investigate Actual locations of underground lines and utilities BEFORE excavating and advise the Architect of all interferences.

3.0 FOUNDATION

- 3.1 The foundation design is based on the recommendations contained in the Geotechnical report by: Sailors Engineering Associates, Inc.
Project Number: 241-039
Date: March 12, 2024
Contact: Jim D. Sailors, PE
Phone Number: (770) 962-9922
- 3.2 Assumed safe bearing capacities listed below shall be confirmed in the field by a Registered Geotechnical Engineer, approved by the Architect.
Continuous Footings 3000 P.S.F.
Isolated Spread Footings 3000 P.S.F.
- 3.3 All foundations shall bear below the frost depth of 18".
- 3.4 Foundations are designed to bear on firm compacted earth or approved Engineered Fill in accordance with the recommendations in the geotechnical report. Where unacceptable material occurs, Excavate and Replace with Engineered Fill. Fill material shall extend 10'-0" (Coordinate with Geotech Recommendations) beyond exterior boundary of building or a distance equal to the footing bearing depth, whichever is greater.
- 3.5 Proof-Roll the areas under the structure with a minimum of with a fully loaded tandem axle dump truck. Repair soft areas per the geotechnical report.
- 3.6 Where compacted earth fill is shown on the Contract Documents, it shall be placed in strict accordance with the geotechnical report. Fill materials shall be placed in 8" lifts and compacted with 30,000 lb vibratory compactor to 95% of ASTM D698.
- 3.7 No footing shall bear directly on rock. Where rock is closer than (2) feet from the bottom of the footing, it shall be undercut to a minimum of (2) feet below the footings and replaced by Engineered Fill. Conform to Project Specifications. When Foundation Bearing Stratum transitions from soil to rock, notify the Architect for special transition instructions.
- 3.8 Foundation or Retaining Walls shall be backfilled with stone conforming to ASTM D448, Size 6.
- 3.9 Foundation Walls not designed as Cantilevered from the footing, shall not be backfilled until shored or permanently supported at top of wall.
- 3.10 Lateral Design soil pressure for Basement Walls is 50 pcf, with an additional surcharge as required.
- 3.11 Lateral Design soil pressure for Cantilevered Retaining Walls is 35 pcf, with an additional surcharge as required.
- 3.12 Provide a drain system for Foundation and Retaining Walls that are a part of the Structure.
- 3.13 Provide 2" diameter Weep Holes at 4'-0" o.c. Maximum in Exterior Retaining Walls. Provide Filter Fabric over the Weep Hole to retain the backfill material.
- 3.14 Where Foundation excavations must remain open and are subject to rain, undercut the excavation and install a (3) inch thick Mud Mat of 2500 psi concrete to protect the bearing soils.

4.0 REINFORCEMENT

- 4.1 Reinforcing Bars shall conform to ASTM A615, Grade 60. Welded Wire Fabric shall conform to ASTM A185.
- 4.2 Detailing of Concrete Reinforcement and Accessories shall be in accordance with ACI "Detailing Manual", SP-86, the CRSI "Manual of Standard Practice" and ACI 318.
- 4.3 Reinforcing shall Not be Heated or Welded.
- 4.4 Reinforcing Placement shall be Approved by the Architect or His/Her Authorized Representative before Concrete is Placed.
- 4.5 Provide the following Concrete Cover for Reinforcement (Exposed Members are Against Exposed to Weather or Earth in Service):
- | MARK | NOT EXPOSED | EXPOSED | CAST AGAINST EARTH |
|-----------------------|-------------|---------|--------------------|
| BEAMS | 1-1/2" | 1-1/2" | --- |
| FOOTINGS | --- | 2" | 3" |
| WALLS - #5 OR SMALLER | 3/4" | 1 1/2" | --- |
| WALLS - #6 OR LARGER | 3/4" | 2" | --- |
- 4.6 Reinforcement for Slabs-On-Grade shall be as noted on the drawings.
- 4.7 Masonry Reinforcement shall be placed in the Center of the Wall, Unless Specifically Detailed Otherwise in these Drawings.
- 4.8 Bars designated Continuous or Bars required to be Spliced for Placement, shall be Lapped as follows:
- Concrete Reinforcement: Class "B" Tension Lap
- Masonry Reinforcement:
- | MASONRY LAP SPLICE LENGTH | | | | | |
|---------------------------|-----|-----|-----|-----|-----|
| BAR SIZE | #4 | #5 | #6 | #7 | #8 |
| 8" CMU | 36" | 45" | 54" | 63" | 72" |
| 12" CMU | 36" | 45" | 54" | 63" | 72" |
- 4.9 Openings in Concrete Slabs and Walls shall be Reinforced on each side of the Opening with Two (2) #6 Bars in each face. Bars shall extend (2) feet beyond opening. Provide Two (2) #4 Diagonal Bars 4'-0" long in each face. All Reinforcement in Slabs and Walls:
4.10 Welded Wire Fabric (WWF) shall lap Two (2) Full Meshes and be Securely Wired at Each Side and End.
4.11 Welded Wire Fabric shall be Fabricated from Sheets. Rolls are Not Allowed.
4.12 Provision Bars at All Wall and Bond Beam Corners. Bars shall be the same size and spacing as Horizontal Reinforcing in each direction.
- 4.13 Intersecting Walls, If Poured separately, shall be Kept and Doweled together with Bars of same size and spacing as Horizontal Reinforcement.

5.0 CONCRETE

- 5.1 All Concrete Workmanship and Materials shall conform to ACI 318 and All Local Codes, Laws and Ordinances.
- 5.2 The following Table shall apply to All Concrete Mix Designs to be used for this Project:
- | TYPE OF CONSTRUCTION | MIN. COMP. STRENGTH AT 28 DAYS (psi) | TOTAL AIR | MAXIMUM W/C RATIO | SLUMP (in.) |
|-----------------------------|--------------------------------------|---------------|-------------------|-------------|
| FOOTINGS AND FDN. WALLS | 3000 | NOT REQ'D. | 0.50 | 4-6 |
| INTERIOR SLAB-ON-GRADE | SEE PLANS | NOT ENTRAINED | 0.50 | 3-5 |
| CONCRETE EXPOSED TO WEATHER | 4000 | 5 - 7% | 0.45 | 4-6 |
| ALL OTHER CONCRETE | 3000 | AS REQ'D. | 0.58 | 4-6 |
- 5.3 Grout used in under column base plates shall be cement based, non-shrink, non-metallic grout. The grout shall exhibit no shrinkage in accordance with ASTM C827. "Test Method for Early Volume Change of Cementitious Mortars" and shall have a minimum 28-day compressive strength of 5000 psi when tested in accordance with ASTM C109, "Test Method for Compressive Strength of Hydraulic Cement Mortars".
- 5.4 Provide entrained air per ACI 318.
- 5.5 Where excess water is added to the concrete so that its serviceability is degraded, the attainment of required strength shall not release the contractor from providing such modifications as may be required by the architect to provide a serviceable member.
- 5.6 All concrete shall be vibrated.
- 5.7 No repair or rubbing of concrete surfaces shall be made prior to inspection by and with approval of the architect (owner or his authorized representative).
- 5.8 Sawed control joint in slab-on-grade shall be cut in accordance with ACI 302.1R. Joints shall be cut within 12 hours of slab placement. The length-to-width ratios of slab areas shall not exceed 1.25. The maximum area of slab within joints shall be 225 s.f.
- 5.9 Provide concrete walls with dovetail anchors and anchor slots where masonry walls abut concrete surface.
- 5.10 Control joints in all foundation and retaining walls shall be placed not more than 20 feet apart and shall have 3/4 inch v-chamfers on each side. Construction joints shall be placed not more than 100 feet apart and shall fall at control joint locations. Construction joints shall be keyed. Discontinue wall reinforcing at control joints. Use 1/2" diameter x 3'-0" smooth bars at 12" c/c centered in the wall. Expansion joints shall be spaced at 150 feet. See details on the drawings.
- 5.11 All pipe penetrations through slabs shall be sleeved in conformance with ACI 318, Section 6.3.
- 5.12 Refer to Civil Drawings for site concrete.
- 5.13 Refer to Drawings of other Disciplines and Vendor Drawings for embedded items and recesses not shown on Structural Drawings.

6.0 STRUCTURAL STEEL AND METAL FORM / DECK

- 6.1 All structural steel work shall conform to the code adopted edition of the AISC 360.
- 6.2 All structural steel, except for plates, angles, channels, pipe and tubing (HSS), shall conform to ASTM A992, Grade 50 (Fy=50 ksi).
- 6.3 Structural steel plates, angles and channels shall conform to ASTM A36.
- 6.4 Steel pipe shall conform to ASTM A53, Grade B.
- 6.5 Square and rectangular structural tubing (HSS) shall conform to ASTM A500, Grade C (Fy=50 ksi).
- 6.6 Anchor bolts shall be ASTM F1554 headed bolts.
- 6.7 All bolted connections shall be made with 3/4 inch diameter ASTM A325 bolts, unless noted otherwise.
- 6.8 The structural steel fabricator shall be responsible for the design of all connections not detailed on the drawings. The fabricator shall provide certification by a registered professional engineer in the project state that the connection design is in accordance with all applicable codes and specifications.
- 6.10 Fabrication and erection shall be done by steel fabricator and erector who have been certified by the aisc quality certification program, category CnBg, or have an independent testing laboratory approved by the architect certify that the fabrication procedures used in this work are in accordance with AISC specifications and these requirements.
- 6.11 Welding shall be done by certified welders using ASTM E70 series electrodes for shop welding A36 steel, and E70 series low hydrogen electrodes for all welding of high strength steels and for all field welding.
- 6.12 Welds shown on structural drawings are minimum design requirements. The fabricator's shop drawings shall reflect welds in accordance with AWS requirements.
- 6.13 A minimum representative sampling of 10% of all fillet welds by each welder shall be visually inspected.
- 6.14 A representative sampling of full penetration groove welds shall be inspected by ultrasonic testing. Twenty-five percent of the welds by each welder shall be inspected. See specifications for additional requirements.
- 6.15 When welds are not called-out on drawings, they are minimum size continuous fillet welds in accordance with AWS D1.1. Fillet welds not specified as to length shall be continuous.
- 6.16 All groove welds shall be full penetration.
- 6.17 Provide fillet welds at all contact joints between steel members sufficient to develop the allowable tensile strength of the smaller member at the joint, UNO.
- 6.18 Metal form deck and roof deck shall be continuous over three (3) spans and installed in accordance with the manufacturer's recommendations. See specifications for additional requirements.
- 6.19 Metal deck is designed for uniform loads on the spans shown. No concentrated point or line loads shall be induced on metal deck.
- 6.20 The contractor is responsible for the provision of a method to transfer gravity and lateral loads from non-structural items occurring between structural framing to adjacent framing members. If structural framing consists of joists or joist girders, special provisions apply. See steel joist and joist girder notes for further information.
- 6.21 Coordinate all openings and dimensions with Architectural and Mechanical drawings. Field confirm all dimensions.
- 6.22 Protect all steel below grade by encasing in concrete or painting with bituminous paint.
- 6.23 Galvanizing of steel members shall conform to ASTM A123.
- 6.24 Headed studs used in fabrication of embedded assemblies in concrete shall conform to ASTM A108 and shall be welded to those assemblies using a full fusion process.
- 6.25 In all cases, support of metal deck around column closure, screed plates around the openings and edges of slabs shall be provided by the contractor.
- 6.26 Hardened washers shall be installed over short slotted or oversize holes occurring in the outer ply of a connection. A plate washer of at least 5/16 inch thickness with standard holes shall be installed over long slotted holes occurring in an outer ply of a connection.
- 6.27 Base plate connections are not designed to provide stability of columns during erection. Columns shall be temporarily braced by the erector prior to release of the column from the hoisting equipment.
- 6.28 Steel encased in concrete shall not be painted.
- 6.29 Column anchor bolt holes shall be oversized in accordance with AISC "Manual of Steel Construction, Volume II, Connections".

7.0 MASONRY

- 7.1 All Masonry Construction shall comply with ACI 530, "Building Code Requirements for Concrete Masonry Structures".
- 7.2 Masonry for this Structure has designed in accordance with the Referenced Code as Inspected Masonry. The Owner shall employ an Agent in Compliance with Code Criteria to insure that the Code requirements are carried out. Inspection and Testing shall conform to ACI 530 Section 1.5.
- 7.3 Minimum Compressive Strength of Concrete Masonry at 28 days shall be F'm = 2,000 psi. All Load Bearing block masonry shall have a minimum Net Area unit strength of 2,000 psi at 28 days.
- 7.4 Concrete Masonry Units shall conform to ASTM C90 or ASTM C55 and be sampled and tested in accordance with ASTM C140.
- 7.5 Bed Joint thickness shall not exceed 5/8".
- 7.6 Grout used for filling Cells and Bond Beams shall comply with ASTM C476 and shall have a Minimum Compressive Strength of 2,000 psi at 28 days determined in accordance with ASTM C140. The Slump shall be between 9 inches and 11 inches. Test Gout strength in accordance with ASTM C1019 for each 5000 square feet of wall.
- 7.7 Where the minimum dimension of any continuous vertical cell is 3 inches or less, use Fine Grout, otherwise use Course (Pea Gravel) Grout.
- 7.8 Mortar shall conform to the following Types as defined in the Building Code:
- Masonry in contact with earth: Type M
- Exterior Block Walls and Bearing Walls: Type M or S
- Non-Bearing Interior Partitions: Type N
- Brick Walls or Brick Veneer: Type N
- 7.9 Provide Control Joints at locations Approved by the Architect in all Masonry Walls at a maximum spacing of 20 feet or 3 times the wall height, whichever is less. Also provide Control Joints adjacent to corners, at changes in wall height and at changes in foundation conditions. Provide a Control Joint at Masonry Walls where support changes from Continuous Strip Footings to thickened slab-on-grade.
- 7.10 Joint Reinforcement shall be discontinued at control joints and where masonry veneer is supported from the Structure.
- 7.11 Do Not locate Control Joints within 16 inches of openings.
- 7.12 Horizontal Bond Beam reinforcement shall run continuous through Control Joints.
- 7.13 Joint Reinforcement must meet ASTM A82. Provide the following Minimum Continuous Horizontal Masonry Reinforcing at 16" O.C. (Manufactured by Du-O-Wal or an approved substitute.)
- Single Wythe:
Unreinforced - Std. Weight, Truss type
Reinforced - 8" width Std. Weight Ladder type
10"12" width - Medium Weight Ladder Type
- Cavity Walls:
Unreinforced - Std. Weight Rectangular Tab Tie
Reinforced - 8" width - Std. Weight Rectangular Tab Tie
10"12" width - Medium Weight Rectangular Tab Tie

- 7.14 All Reinforced Hollow Unit Masonry shall be built to preserve the Unobstructed Vertical Continuity of the cells to be filled. Walls and cross webs forming such cells to be filled shall be full bedded in Mortar to prevent leakage of the grout. All Head (or end) joints shall be solidly filled with mortar for a distance in from the face of the wall or unit not less than the thickness of the longitudinal face shells. Bond shall be provided by lapping units in successive vertical courses or by equivalent Mechanical anchorage.
- 7.15 Vertical Cells to be filled shall have Vertical Alignment sufficient to maintain a clear, unobstructed, continuous vertical cell measuring not less than 3 inches and having a clear area of 10 square inches.
- 7.16 Cleanout openings shall be provided at the bottom of all cells to be filled in each pour of grout where such grout pour in excess of (4) four feet in height. Any overhanging mortar or other obstruction or debris shall be removed from the insides of such cell walls. The cleanouts shall be sealed after Inspection and before grouting.
- 7.17 Vertical Reinforcement shall be held in position at Top and Bottom and at intervals not exceeding (192) diameters of the reinforcement or (10) feet.
- 7.18 All Cells containing Reinforcement shall be filled solidly with grout. Grout shall be poured in lifts of (8) feet maximum height. All Grout shall be less than (1) inch or more than (2) inches below the top of the uppermost unit grouted.
- 7.19 When Total Grout Pour exceeds (8) feet in height, the grout shall be placed in (4) foot lifts and Special Inspection during Grouting shall be less than (1) inch or more than (2) inches Below the top of the uppermost unit grouted.
- 7.20 When the Grouting is stopped for (1) one hour or longer, Horizontal Construction joints shall be formed by stopping the pour of grout not less than (1) inch or more than (2) inches Below the top of the uppermost unit grouted.
- 7.21 The Architect shall be given a Minimum of (24) hours notice prior to each reinforced block grouting operation.
- 7.22 Provide adequate bracing for all masonry walls during construction and until lateral supports and diaphragms have been attached and grout has attained the specified Design Strength.
- 7.23 Where Lintels bear on Masonry Walls, they shall bear on either a bond beam course or on cores filled with concrete. All Lintels shall have at least (8") of bearing at each end, unless noted otherwise.
- 7.24 Steel Beams which bear on Masonry Walls shall be Mechanically Anchored and shall bear either on Bond Beams or filled block cores Reinforced with 2#7 Vertical bars and shall bear a minimum of (8"), unless shown otherwise.

8.0 WOOD FRAMING

- 8.1 All dimensional lumber shall be Southern Pine, No. 2 (mc=19 percent) or equivalent unless noted otherwise.
- 8.2 Nailing shall conform to the minimum nailing requirements as set forth in the Building Code, unless noted otherwise.
- 8.3 Connection hardware for structural members shall be galvanized Strong-Tie connectors by the Simpson Company or equivalent. Use all specified fasteners to achieve maximum allowable load value unless noted otherwise. Hardware in contact with or anchored to pressure treated wood shall comply with Simpson ZMax coating or equivalent.
- 8.4 Provide bolt holes in wood that are a minimum of 1/32 to a maximum of 1/16 inch larger in diameter than the bolt diameter. Provide malleable iron washers or equivalent plate washers under all bolt heads or nuts bearing on wood.
- 8.5 Refer the drawings for Standard Details illustrating acceptable parameters for notching or drilling holes in studs and joists. Any notching or drilling that falls outside the boundaries prescribed in these details must be Reviewed and Approved by the Engineer of Record before proceeding.
- 8.6 Wood in contact with concrete shall be Foundation Grade Pressure-Treated Southern Pine or equivalent. All connection hardware and fasteners in contact with Pressure-Treated wood shall be hot-dipped galvanized, stainless steel or with an approved equivalent finish.
- 8.7 Wood I-Joists and PSLLVL beams, columns and headers noted on the drawings shall be manufactured by Weyerhaeuser. Complete Shop Drawings shall be prepared by the manufacturer showing layout and details necessary for determining fit and placement in the building. A complete set of Design Calculations shall be prepared by the manufacturer under the supervision of a Professional Engineer licensed in the Project State for all I-Joists and PSLLVL beams, headers and columns and their connections to the supporting elements.

9.0 POST-INSTALLED ANCHORS, REBAR, & FASTENERS

- 9.1 The products listed in this section are the design basis for this project. Additional requirements are as follows:
a. Post-install anchors, rebar, and/or fasteners only as directed by the construction documents.
b. Product diameter and embedment shall be as shown in the details.
c. Install products in accordance with Manufacturer's Printed Installation Instructions.
d. Prior to installation, contractor shall contact manufacturer's representative for product-specific installation training and a letter shall be submitted to the Engineer-of-Record (EOR) indicating training has taken place.
e. Unless noted otherwise on the contract documents, refer to the project Building Code and/or evaluation report for required special inspections and proof load requirements.
f. Substitution requests for products other than those listed below may be Substituted by the Contractor to the EOR for review. Substitutions will ONLY be considered for products having a Research Report recognizing the product for the appropriate application under the project Building Code. Substitution requests shall include calculations that demonstrate the substituted product is capable of achieving the equivalent performance values of the design basis product.
1. For anchoring into CONCRETE:
1.1 Mechanical Anchors shall have been tested in accordance with ACI 355.2 and ICC-ES AS193 for cracked concrete and seismic applications. Pre-approved products include:
SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-2713)
1.2 Adhesive for Rebar and Anchors shall have been tested in accordance with ACI 355.4 and ICC-ES AC308 for cracked concrete and seismic applications. Design bond strength has been based on cracked conc. ACI 355.4 temperature category B, and installations into dry holed drilled using a hammer drill into concrete that has cured for at least 21 days. Adhesive anchors shall be installed by a certified adhesive anchor installer when required per ACI318-11 § 9.2.2. Installations required certified installers shall be inspected per ACE-11 § 9.2.4. Pre-approved products include:
SIMPSON STRONG-TIE "SET-3G"
2. For anchoring into MASONRY:
2.1 Solid-grouted Concrete Masonry
2.1.1 Mechanical Anchors shall have been tested in accordance with ICC-ES AC01 or ICC-ES AC105. Pre-approved products include:
SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR-1056)
2.1.2 Adhesive for Rebar and Anchors shall have been tested in accordance with ICC-ES AC58. Pre-approved products include:
SIMPSON STRONG-TIE "SET-3G"
- 2.2 Hollow Concrete Masonry
2.2.1 Mechanical Anchors shall have been tested in accordance with ICC-ES AC105. Pre-approved products include:
SIMPSON STRONG-TIE "TITEN-HD"
- 2.2.2 Adhesive for Rebar and Anchors with screen tubes shall have been tested for use in accordance with ICC-ES AC58. The appropriate screen tube shall be used as recommended by the adhesive manufacturer. Pre-approved products include:
SIMPSON STRONG-TIE "SET-3G"

10.0 SPECIAL INSPECTIONS

- Special Inspection is to be provided in addition to the inspections conducted by the Department of Building Safety and shall not be construed to relieve the Owner or his Authorized Agent from requesting the periodic and called inspections required by the Building Code. Special Inspection shall be paid by the Owner, and shall satisfy all inspection requirements of the Building Code.
- REQUIRED SPECIAL INSPECTIONS
1. In addition to the regular inspections, the following items will also require special inspection in accordance with Chapter 17 of the governing Building Code.
A. Post-Installed Anchors in Concrete
B. Soils (Section 1705.6)
C. Structural Masonry (Section 1705.4)
D. Wood Framing
E. Structural Steel (Section 1705.2)
2. Special inspector shall meet the qualifications as stated in the building code and shall perform the duties and responsibilities as outlined in the building code.
3. Special inspector shall meet the requirements of IBC section 1704. Special inspector(s) shall be hired by the owner to perform the required special inspections. The names of persons or firms who are to perform the special inspections shall be forwarded to the building official for approval. The special inspector(s) shall complete and submit all forms required by the building department having jurisdiction.
4. The special inspector(s) shall:
A. Observe the work assigned for conformance to the approved drawing and specifications.
B. Furnish inspection reports to the engineer of record and building department. Discrepancies shall be brought to the immediate attention of the contractor for correction, then, if not corrected to the engineer and the building department.
C. Submit to the engineer of record and the building department a signed final report stating that the work was in conformance with the approved drawings and specifications and the applicable workmanship provisions of the IBC.
5. Special Inspection Notes:
A. Continuous special inspection is always required during the performance of the work unless specifically noted below.
B. Where fabrication of structural load-bearing members and assemblies is being performed on the premises of a fabricator's shop, continuous special inspection is required during the performance of the work except as allowed in IBC section 1704 and unless specifically noted below.
C. It is the responsibility of the contractor to provide the special inspector(s) with advance notice, no less than one working day, of the initiation of any work required to have special inspections. All work performed without required special inspection will be subject to removal.
6. All Special Inspection requirements shown in Chapter 17 of the Building Code must be satisfied in addition to any requirements shown on this sheet.



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SHELL -
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912 N CHANCERY ST
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WARREN COUNTY

PERMIT SET
05.16.24
MJM # 23336

NO. DESCRIPTION DATE

GENERAL
NOTES

S0.0

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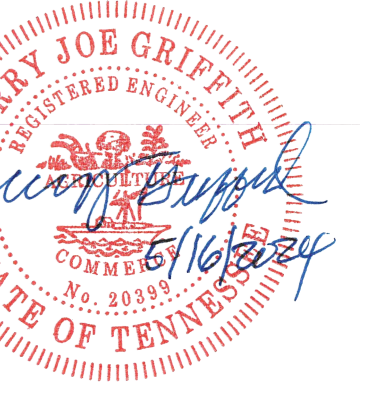
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GENERAL
NOTES

S0.0



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FLETCHER BRIGHT COMPANY

1007 ASHLAND TERRACE
 SUITE 104A
 CHATTANOOGA, TN 37415

OWNER

DOLLAR TREE SHELL - McMINNVILLE

912 N CHANCERY ST
 McMINNVILLE, TN 37110
 WARREN COUNTY

LOCATION

PERMIT SET

05.16.24

MJM # 23336

DATE

NO.	DESCRIPTION	DATE

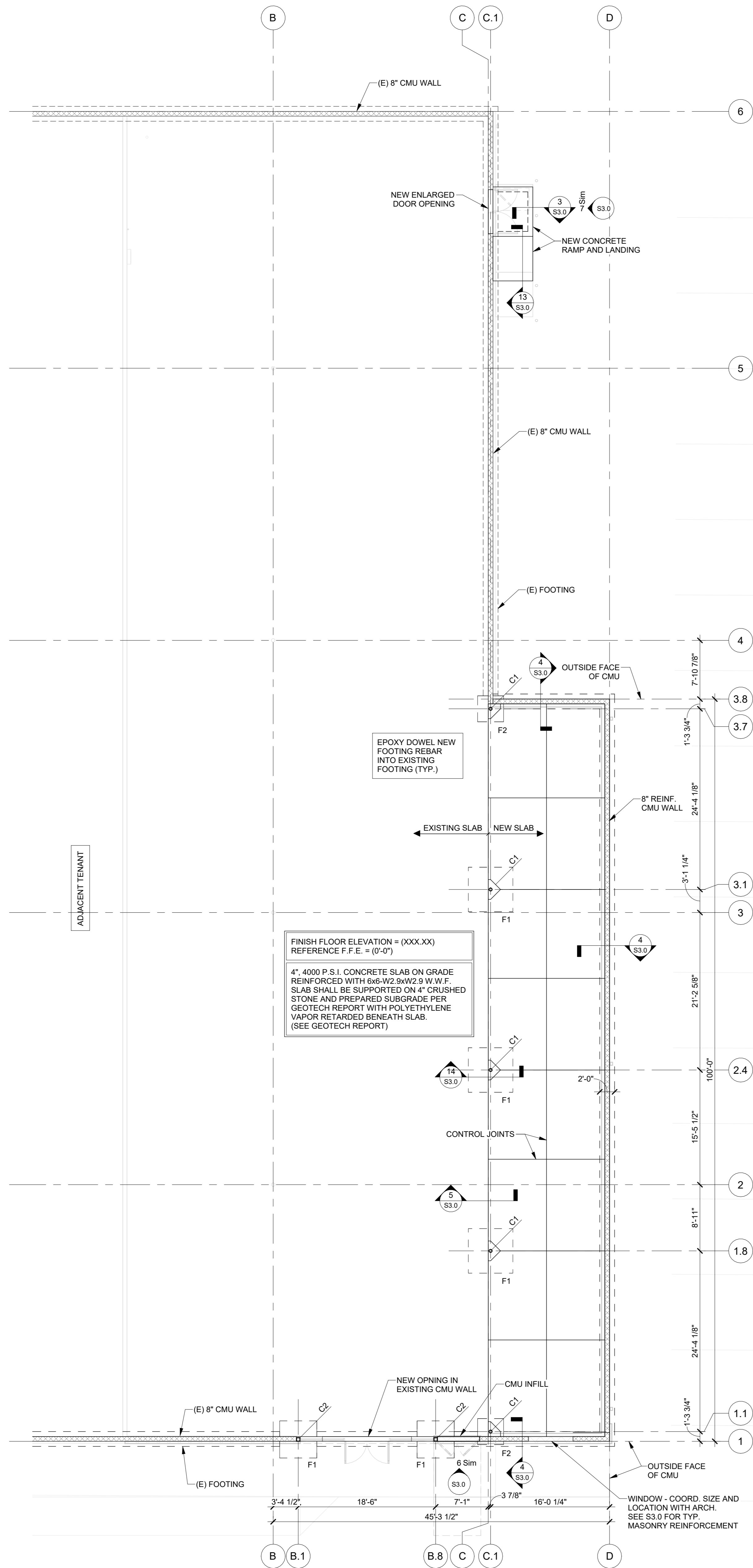
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FOUNDATION PLAN

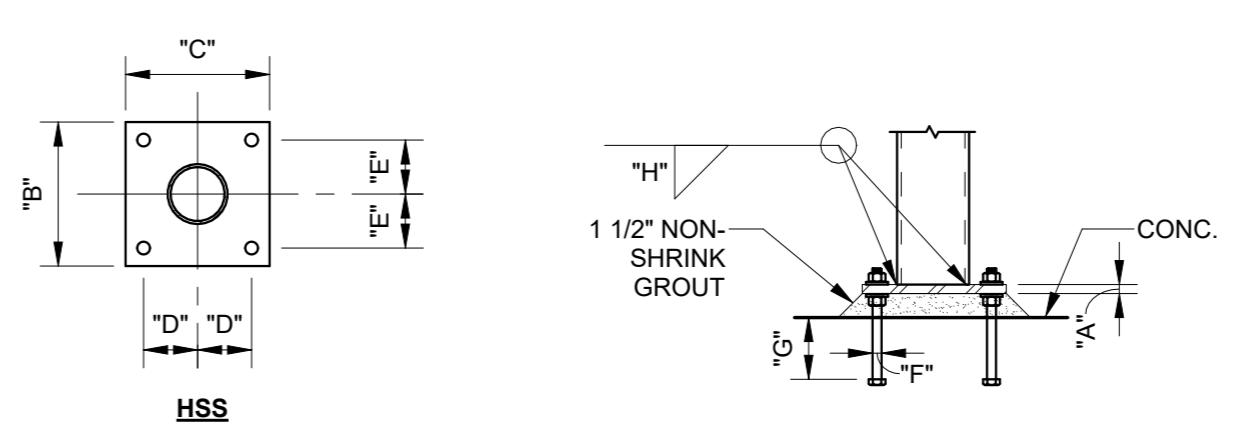
S1.0

SHEET



FOUNDATION PLAN
 SCALE: 1/8" = 1'-0"

- FOUNDATION NOTES:**
- TYPICAL EXTERIOR FOOTING = (1'-4"), UNLESS NOTED OTHERWISE. TYPICAL INTERIOR FOOTINGS = (4'-8"), UNLESS NOTED OTHERWISE. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BEAR AT A MINIMUM OF 1'-6" BELOW FINISH GRADE. BEARING ELEVATION MUST BE ADJUSTED FOR BLOCK COURSING AND VERIFIED IN THE FIELD BASED ON SITE GRADING PLAN. (COORDINATE WITH CIVIL).
 - REFER TO DRAWING S0.0 & S0.1 FOR GENERAL NOTES.
 - SEE 1/S3.0 FOR TYPICAL SLAB-ON-GRADE CONTROL JOINT DETAIL.
 - VERIFY DIMENSIONS WITH ARCHITECTURAL DRAWINGS. DIMENSIONS SHOWN ON ARCHITECTURAL DRAWINGS TAKE PRECEDENCE OVER DIMENSIONS SHOWN ON THIS PLAN.
 - LOWER FOOTINGS WHERE REQUIRED TO ACCOMMODATE PLUMBING, SLAB RECESSES, FINISH GRADE, ETC. REFERENCE ARCHITECTURAL AND PLUMBING DRAWINGS.
 - PROVIDE CONTROL JOINTS IN SLAB-ON-GRADE IN ACCORDANCE WITH GENERAL NOTE 5.8 ON DRAWING S0.0.
 - PROVIDE (2) #4 x 4'-0" DIAGONAL BAR AT ALL SLAB INSIDE CORNERS.
 - NOTES FOOTING STEP WITH TYPICAL TOP/FOOTING ELEVATION NOTED. SEE 3/S3.0 FOR DETAIL. CONTRACTOR TO COORDINATE EXACT LOCATION AND ELEVATIONS WITH FINAL SITE GRADING PLANS.
 - PROVIDE CONTROL JOINTS IN CMU WALL PER NOTE 7.9/S0.0.
 - REFER TO GEOTECH REPORT FOR ADDITIONAL INFORMATION REGARDING SOIL UNDER SLAB AND FOUNDATIONS.
 - FOOTINGS AND BASE PLATES WITH SIDES OF UNEQUAL LENGTH ARE TO BE ORIENTED AS DRAWN ON PLAN.



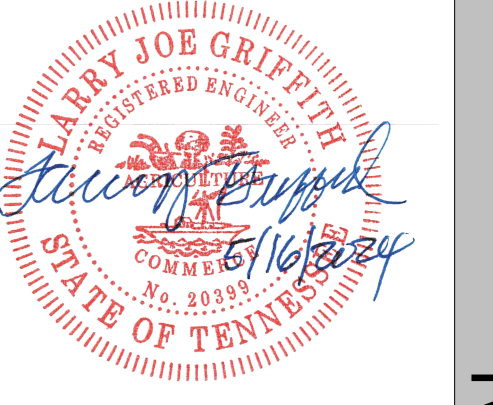
COLUMN MARK	COLUMN SIZE	BASE PLATE					ANCHOR BOLTS **		WELDED	REMARKS	
		A	B	C	D	E	F	G			H
C1	5" DIA. STD. WT. PIPE	3/4"	12"	12"	4 1/2"	4 1/2"	3/4"	9"	4	5/16"	
C2	HSS6x6x3/8	3/4"	12"	12"	4 1/2"	4 1/2"	3/4"	9"	4	5/16"	

** ASTM F 1554 (36ksi) ANCHOR BOLTS (RODS OR HEADED BOLTS) W/ (2) FLAT WASHERS AND (2) HEX NUTS (TYP.)

FOOTING SCHEDULE

MARK	FOOTING SIZE	REINFORCING
F1	5'-0" x 5'-0" x 1'-0"	#5 @ 12" O.C. MAX. E.W.
F2	3'-6" x 3'-6" x 1'-0"	#5 @ 12" O.C. MAX. E.W.

SEE FOUNDATION NOTE 1 REGARDING FOOTING ELEVATIONS



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McMINNVILLE

LOCATION

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WARREN COUNTY

PERMIT SET
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DATE

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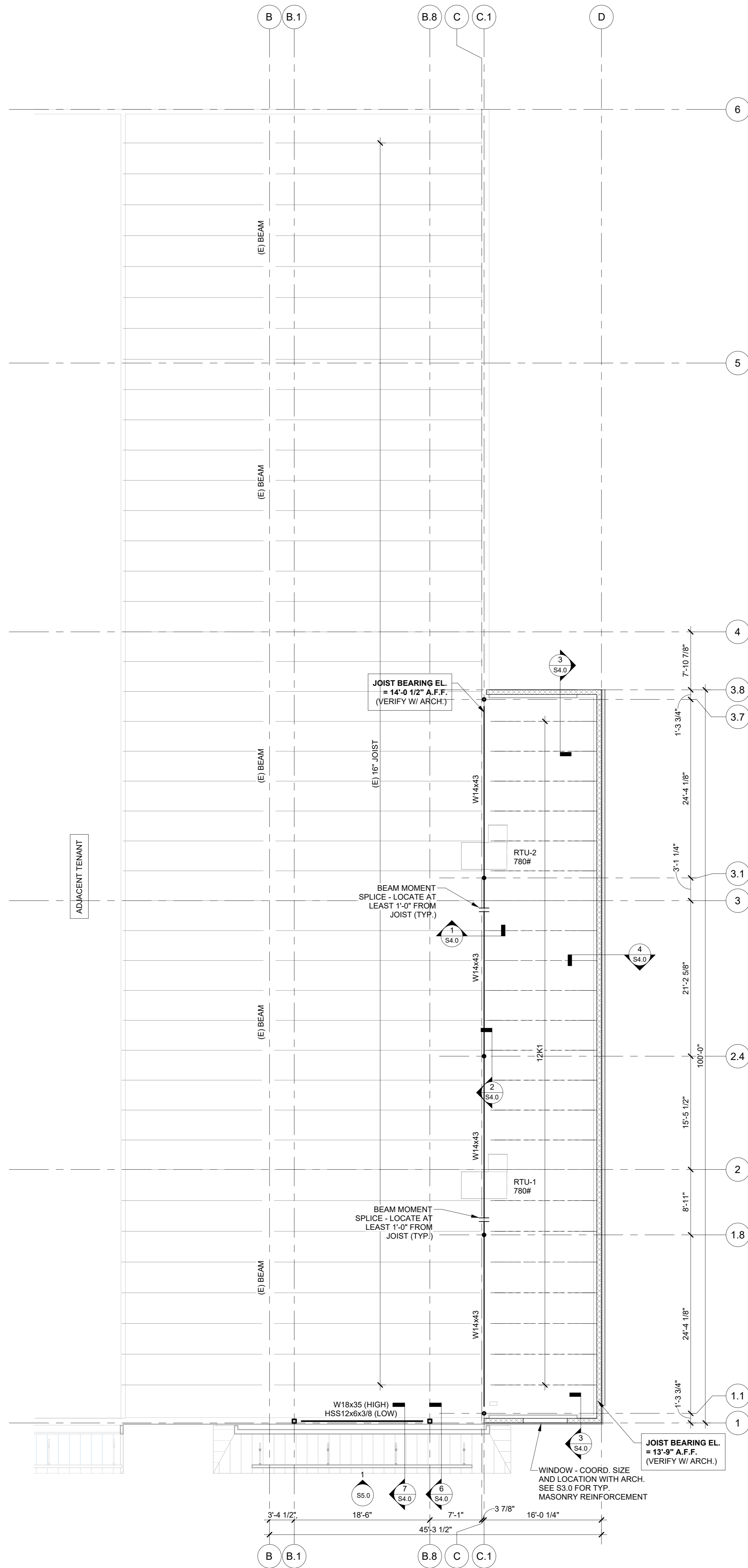
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ROOF FRAMING PLAN

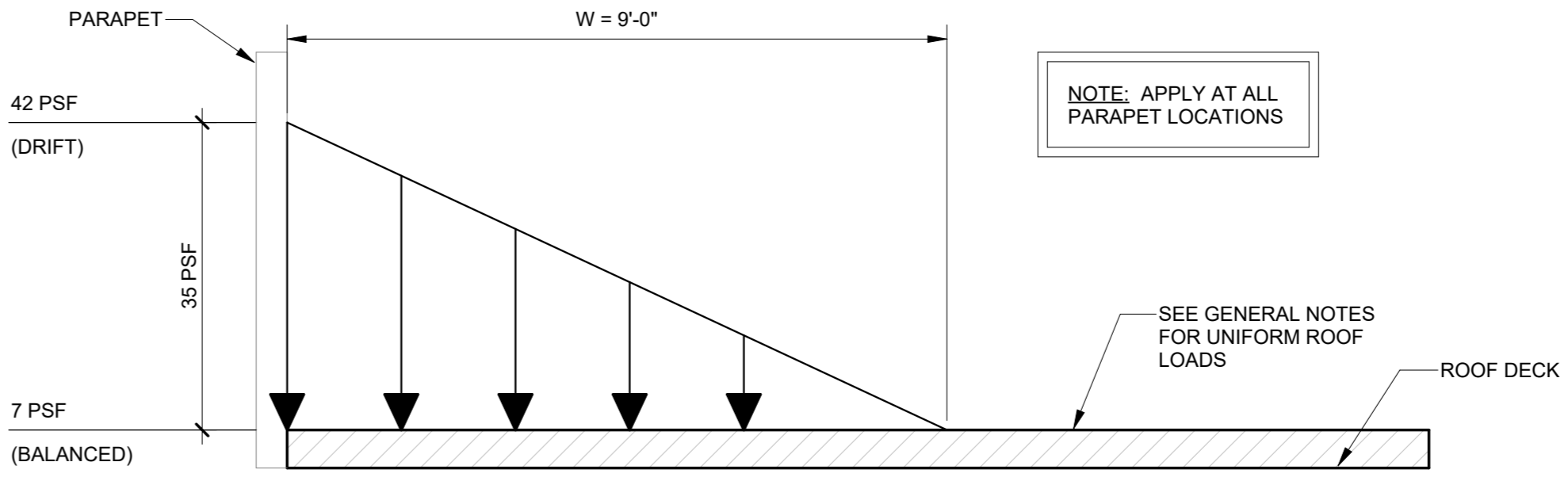
SHEET

S2.0

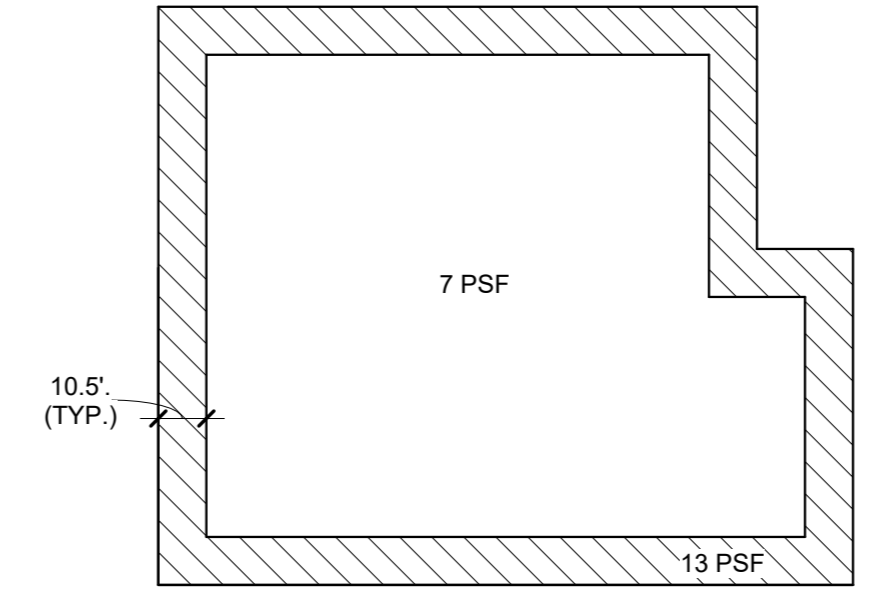


ROOF FRAMING PLAN
SCALE 1/8" = 1'-0"

- FRAMING NOTES:**
- REFER TO DRAWING S0.0 FOR GENERAL NOTES.
 - ALL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT AS SHOWN ON THE JOIST NET UPLIFT PLAN AND FOR SNOW DRIFT AS SHOWN IN THE SNOW DRIFT DIAGRAM.
 - ALL JOISTS SHALL HAVE 2 1/2" BEARING SEATS, UNLESS NOTED OTHERWISE.
 - ROOF DECK SHALL BE 1 1/2" TYPE B, 22 GAGE, PRIMED.
 - BRIDGING FOR JOISTS SHALL BE SIZED AND LOCATED BY THE JOIST MANUFACTURER IN ACCORDANCE WITH SJI REQUIREMENTS.
 - SEE S4.0 - S4.1 FOR TYPICAL FRAMING DETAILS.
 - REFER TO S5/S4.0 FOR ATTACHMENT OF STEEL ROOF DECK.
 - JOISTS SHALL BE SPACED @ 6'-0" O.C. MAX. (TYPICAL - U.N.O.)
 - VERIFY ALL DIMENSIONS & ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DIMENSIONS SHOWN ON ARCHITECTURAL DRAWINGS TAKE PRECEDENCE OVER DIMENSIONS SHOWN ON THIS PLAN. NOTIFY EOR OF ANY DISCREPANCIES.
 - RTU PLACEMENT PARAMETERS:**
-TWO RTUs MAY NOT SHARE A SUPPORTING JOIST
-MAXIMUM ALLOWABLE RTU WEIGHT IS 2000 LB
 - GROUT SOLID. ALL REINFORCED CELLS IN CMU WALLS.
 - INDICATES MOMENT CONNECTION.



SNOW DRIFT LOADS
N.T.S.



JOIST NET UPLIFT PLAN
(NOMINAL PRESSURES)



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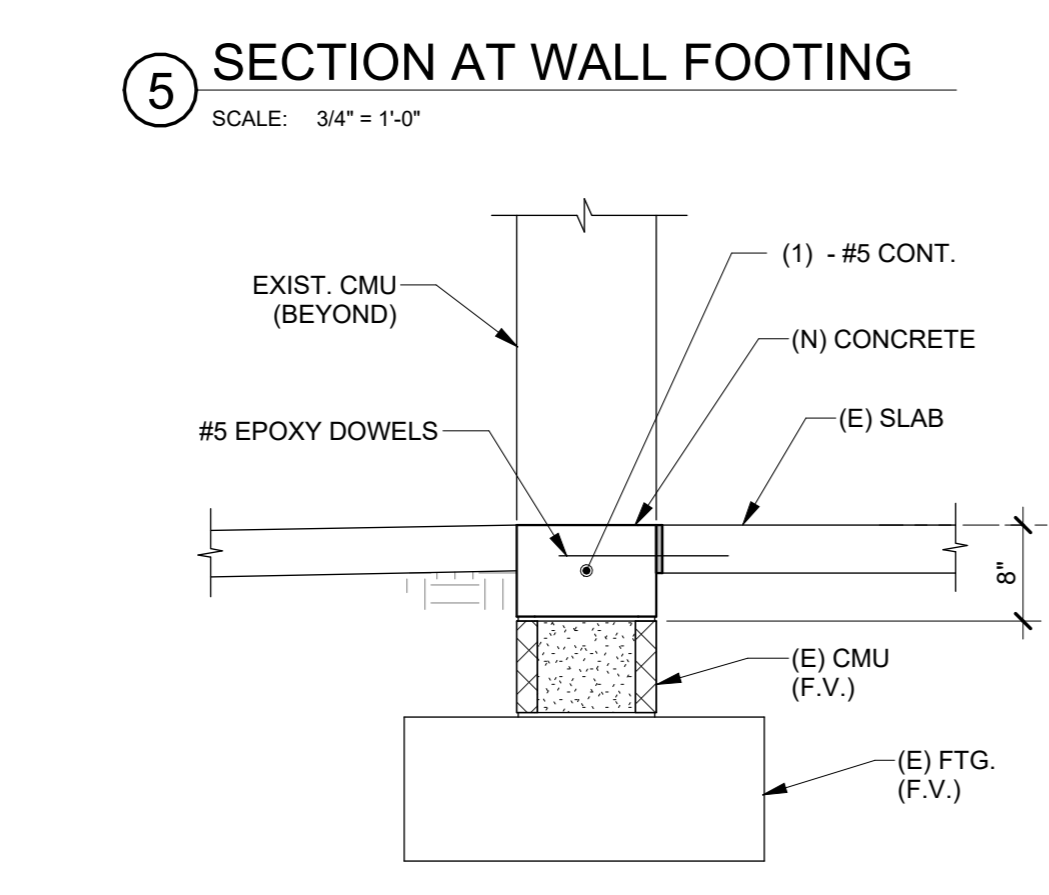
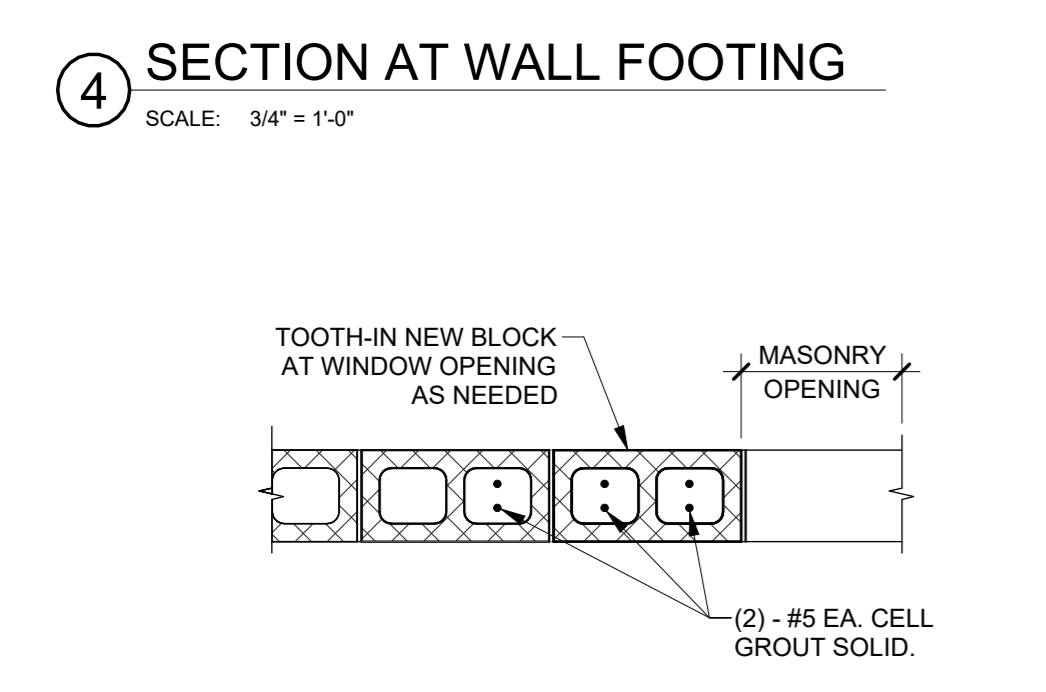
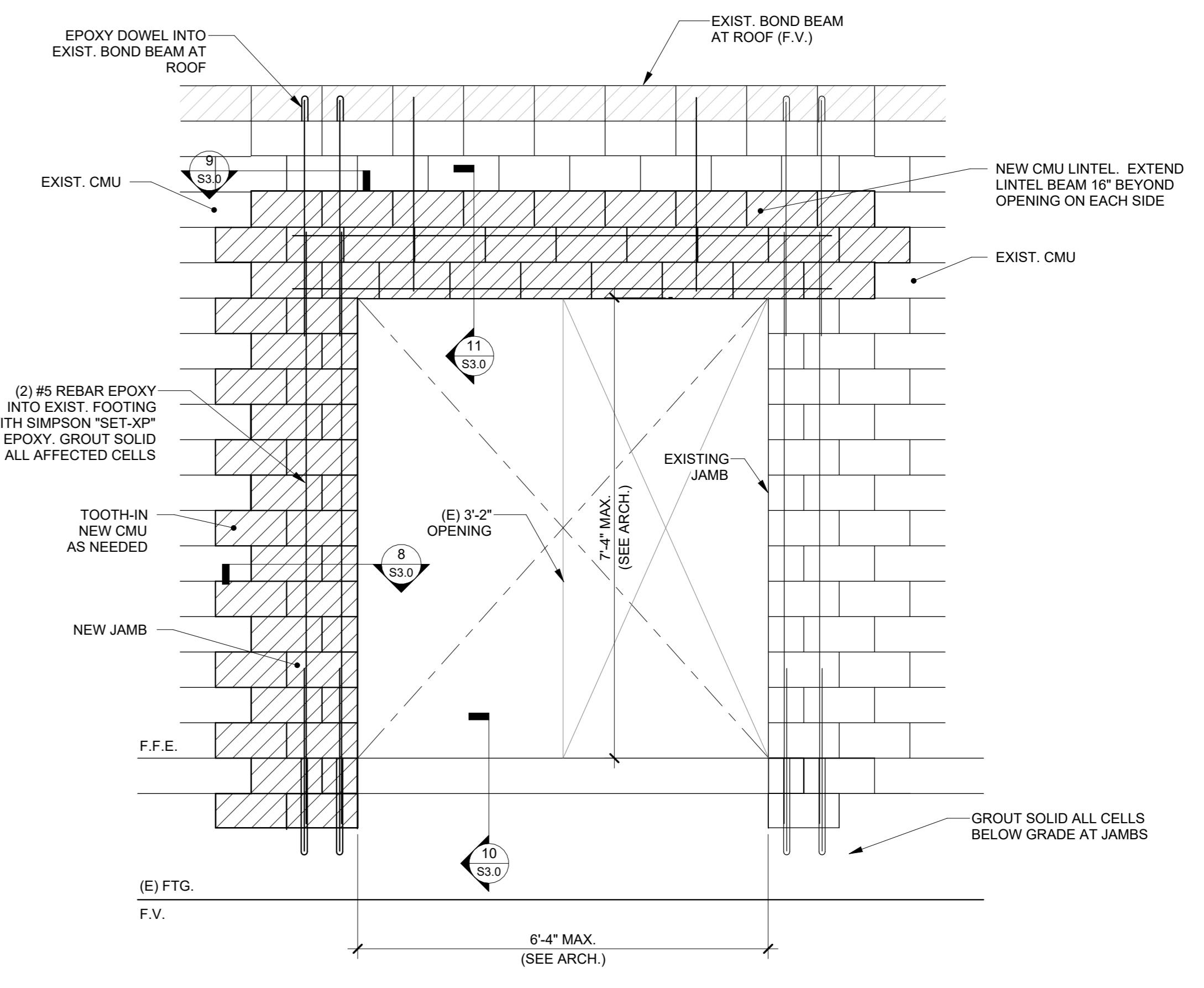
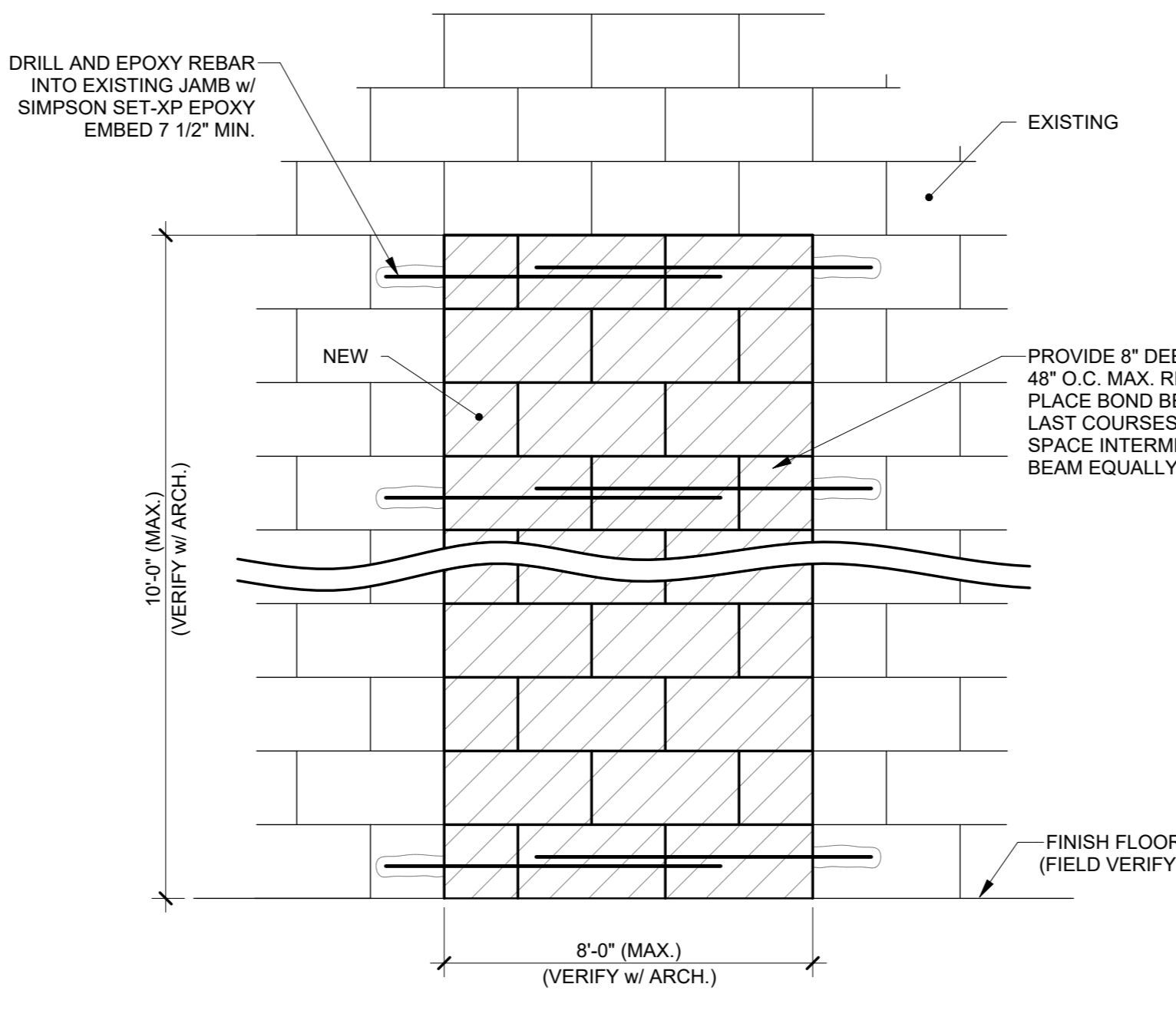
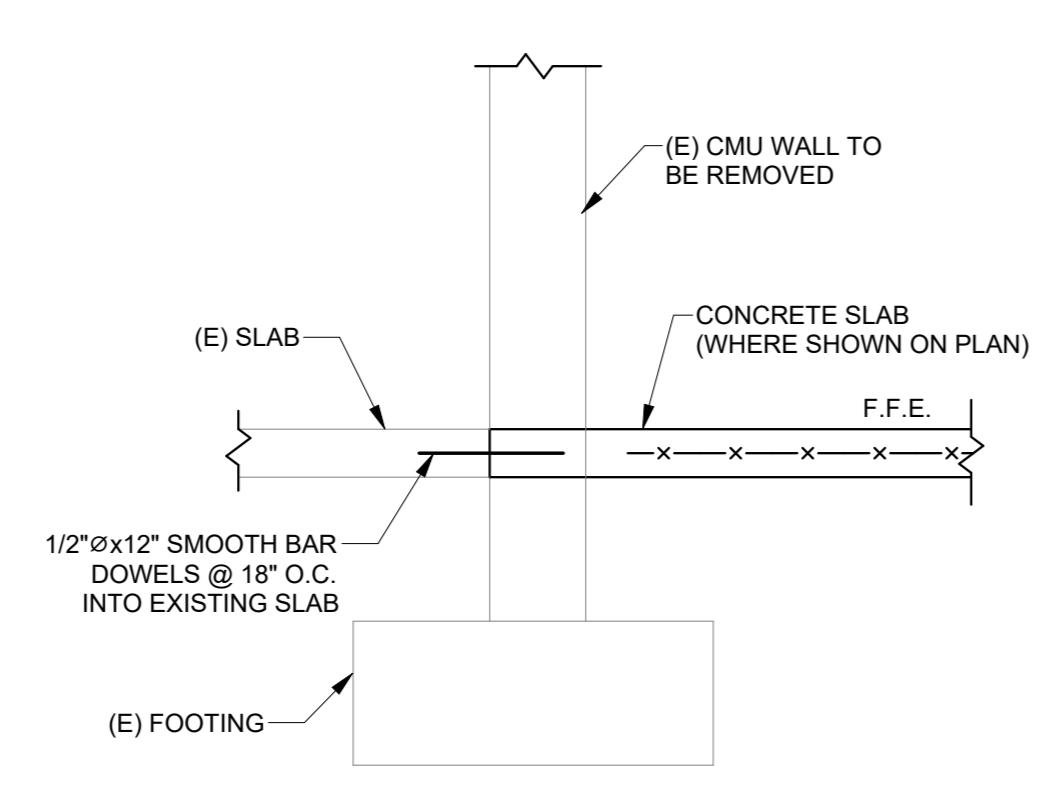
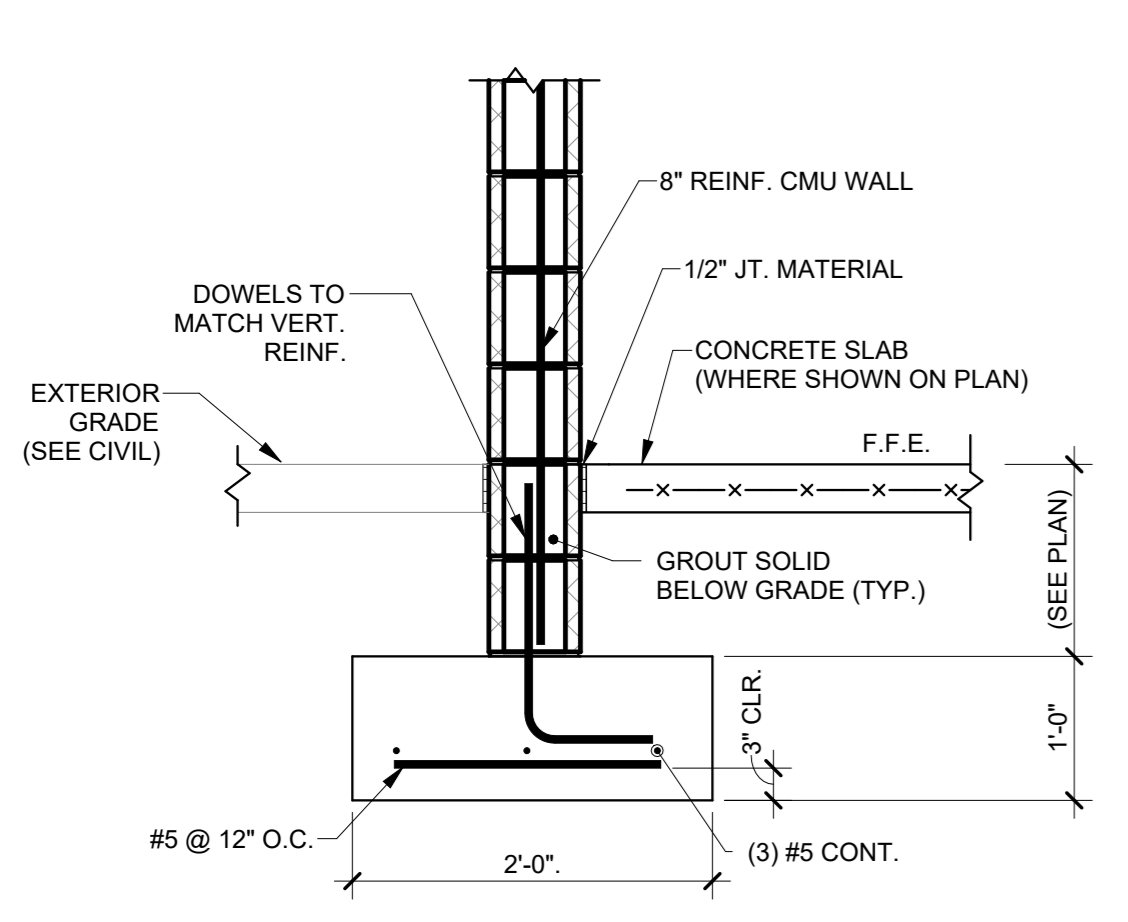
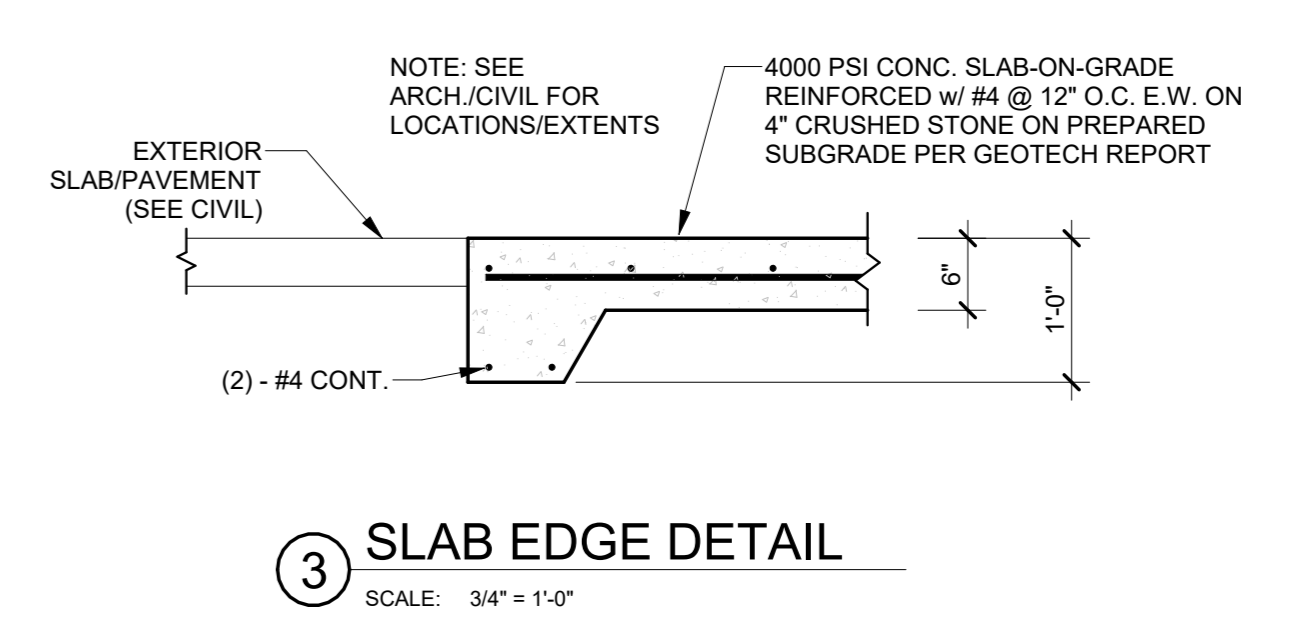
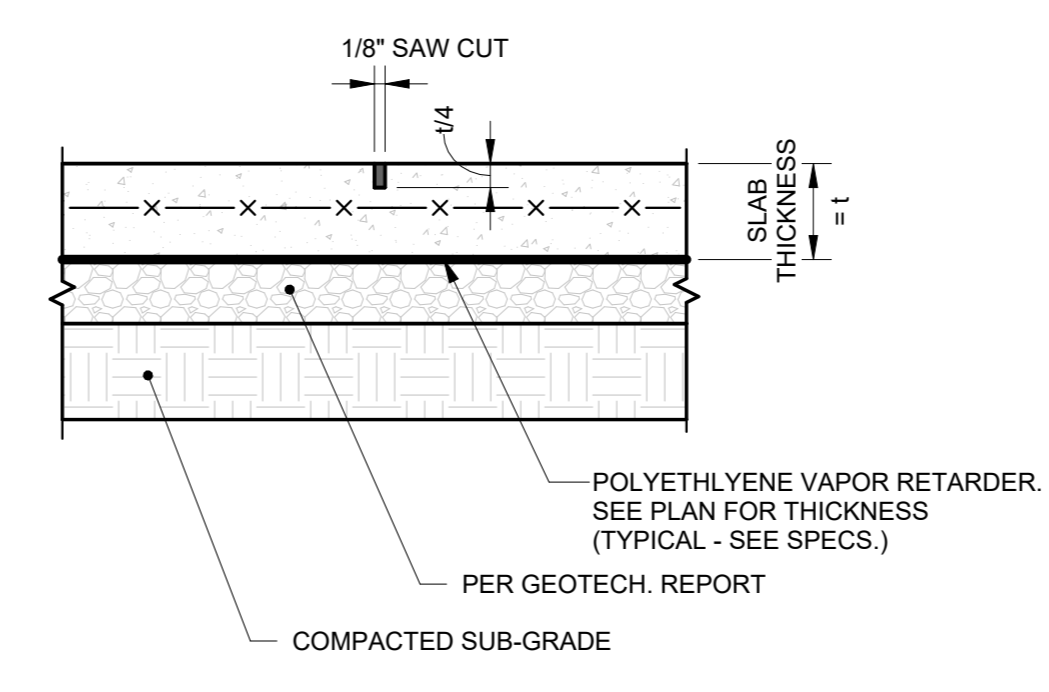
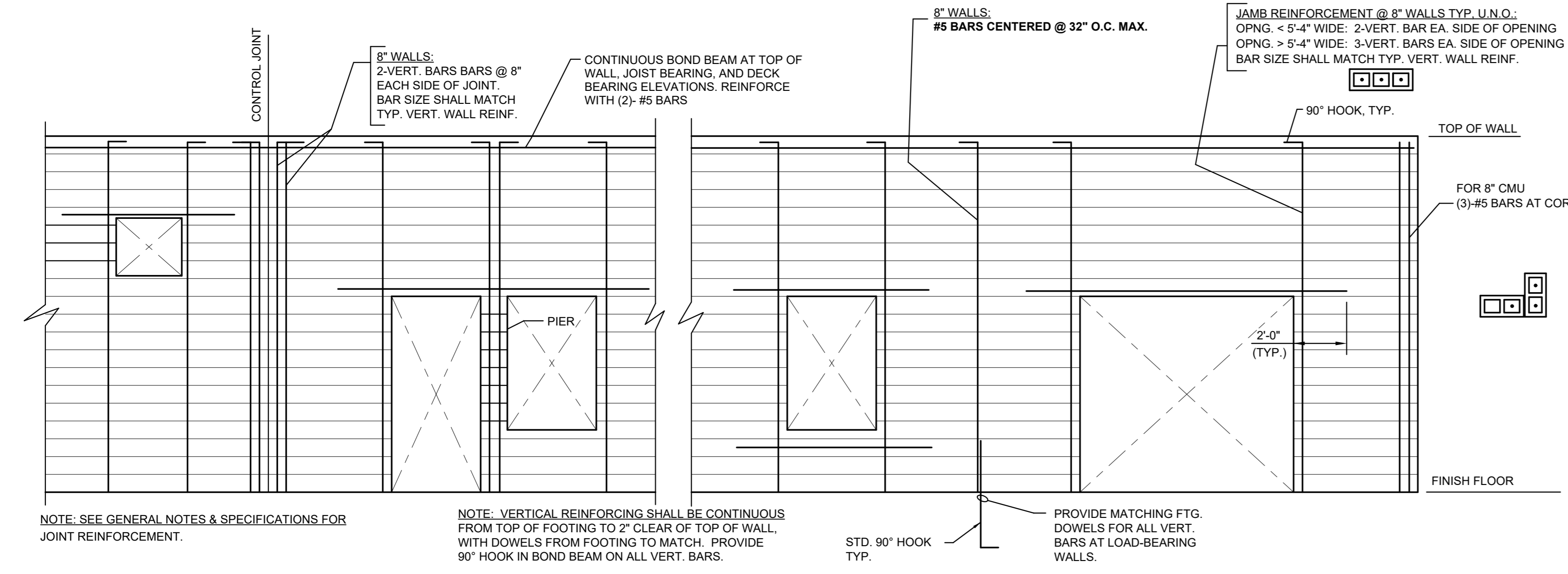
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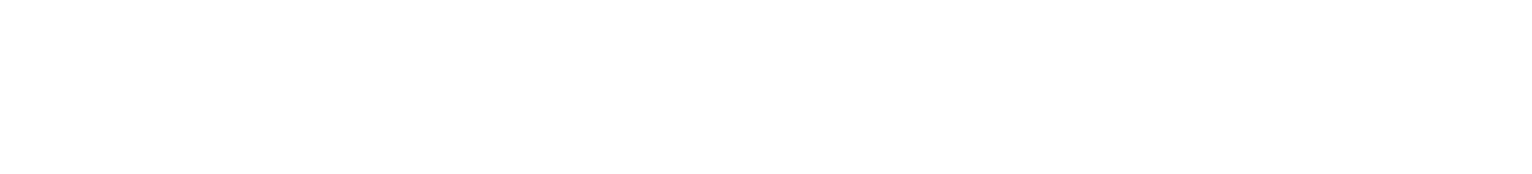
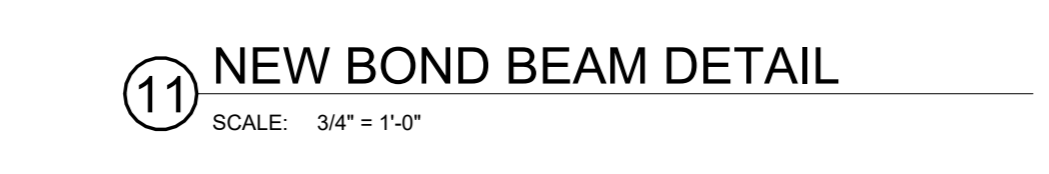
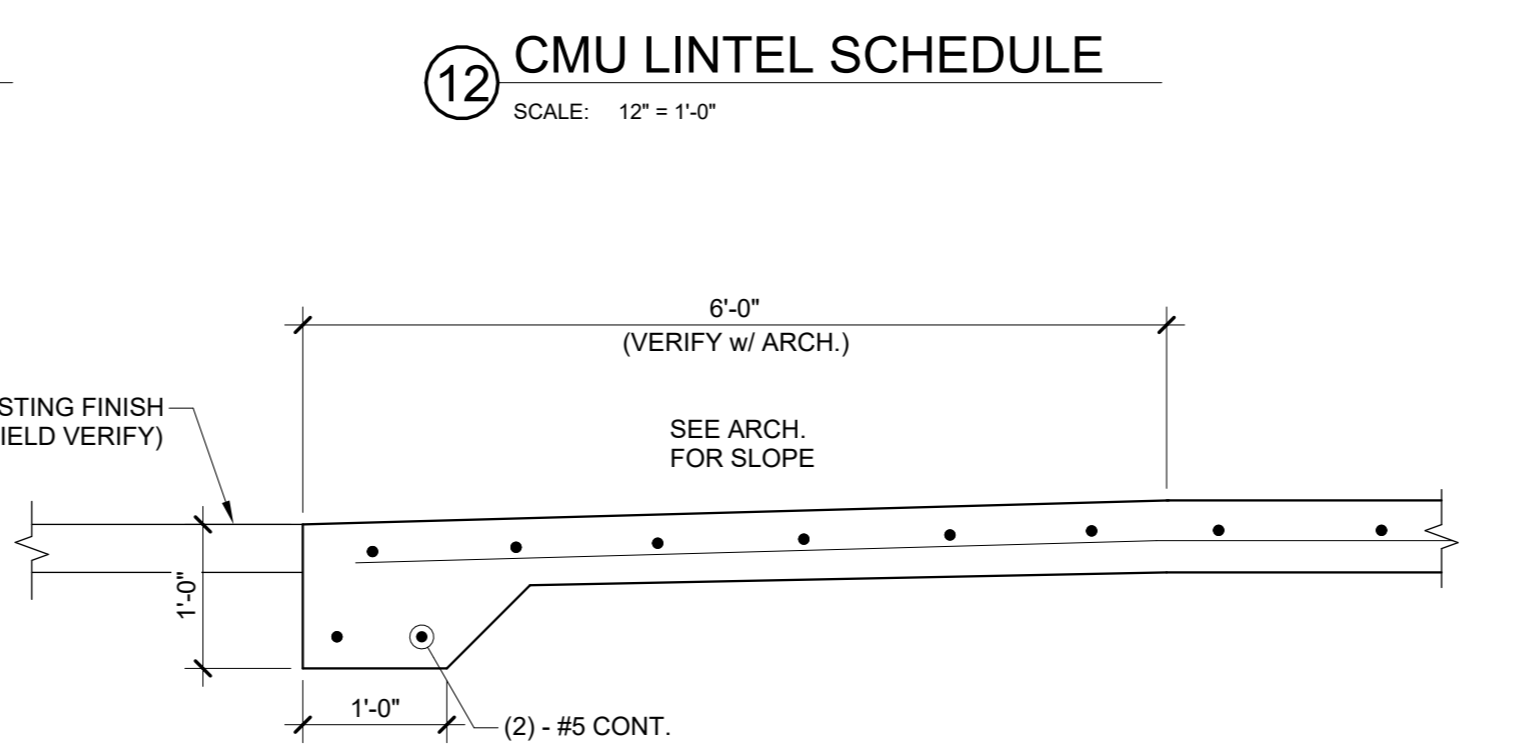
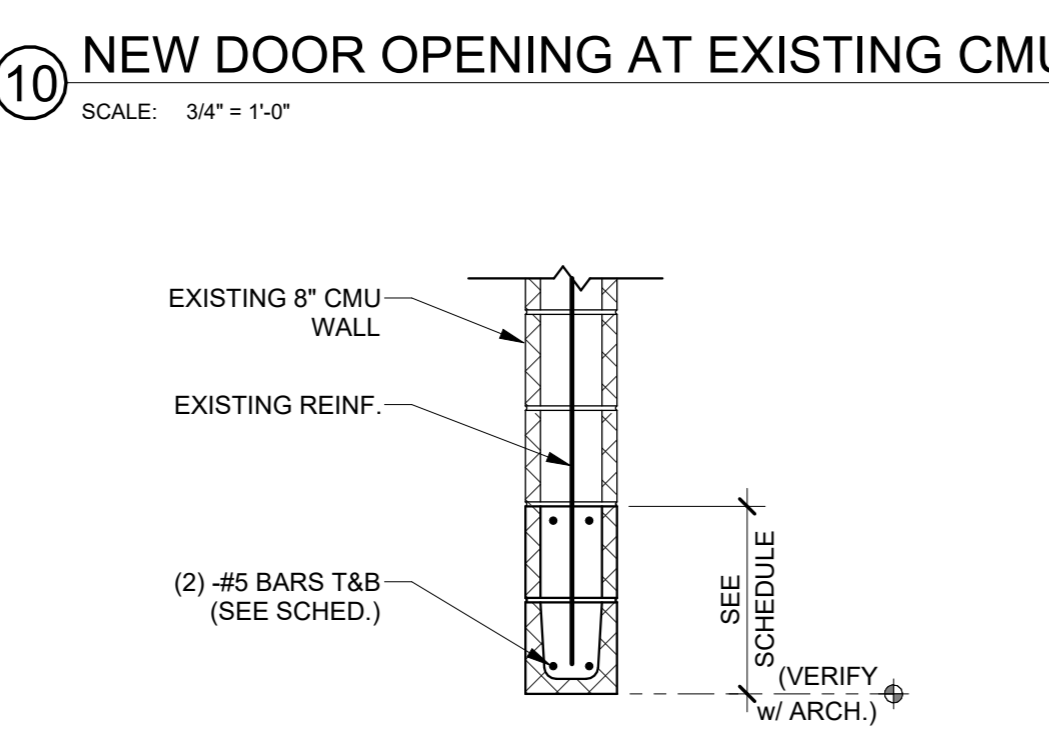
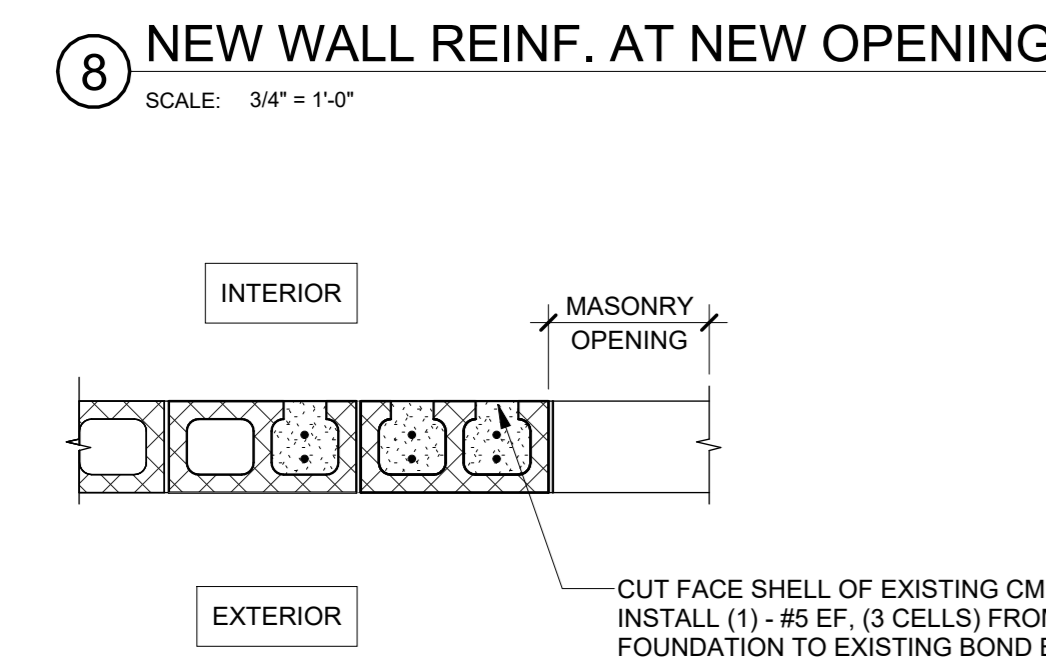
FOUNDATION
DETAILS

S3.0



CMU LINTEL SCHEDULE
MINIMUM BEARING 8" EACH END

WALL SIZE	LINTEL TYPE
8" BLOCK	8" X 16" BOND BEAM W/ (2) - #5 TOP & BOT.
OPENINGS FROM 4'-0" TO 6'-8"	
WALL SIZE	LINTEL TYPE
8" BLOCK	8" X 24" BOND BEAM W/ (2) - #5 TOP & BOT.





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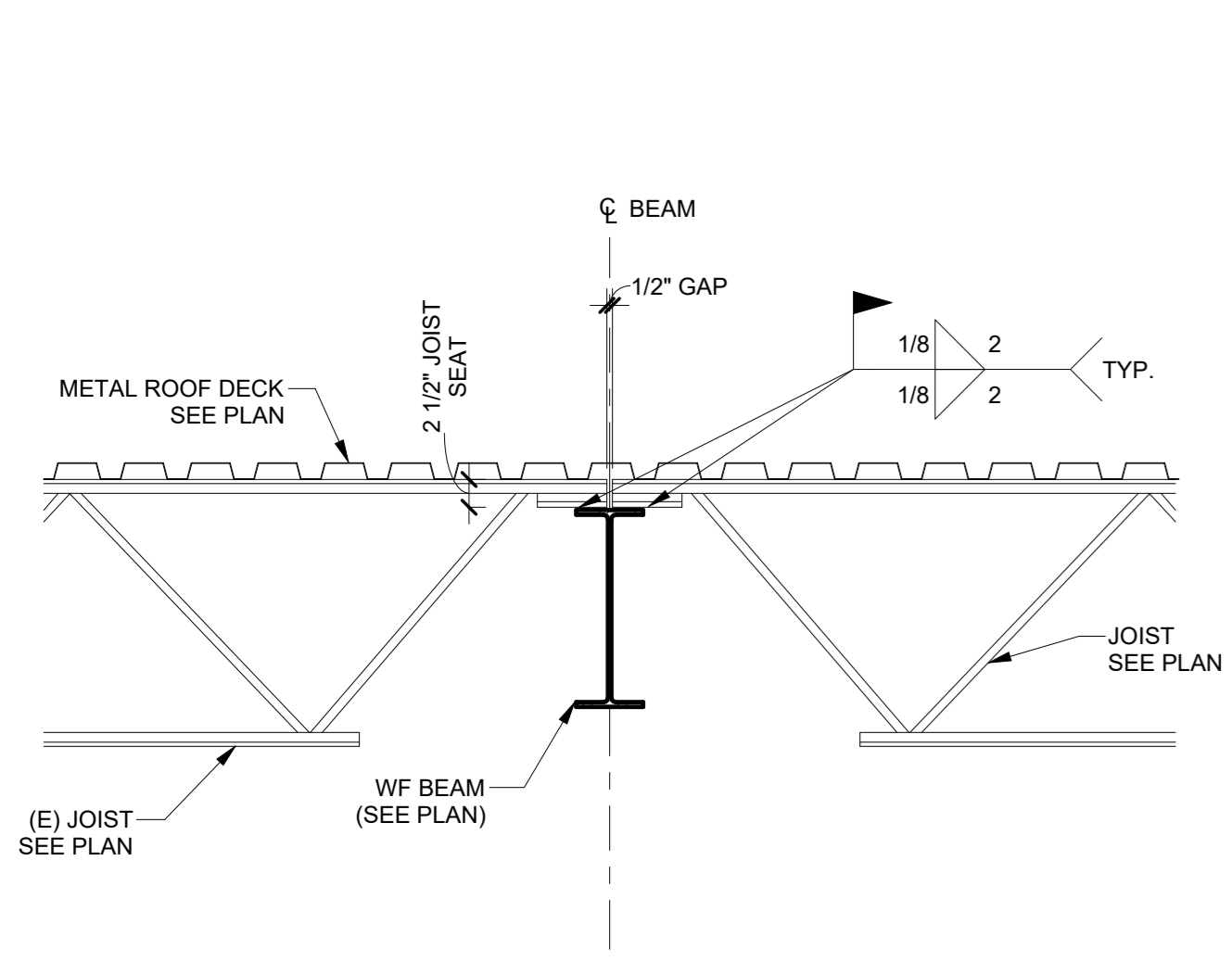
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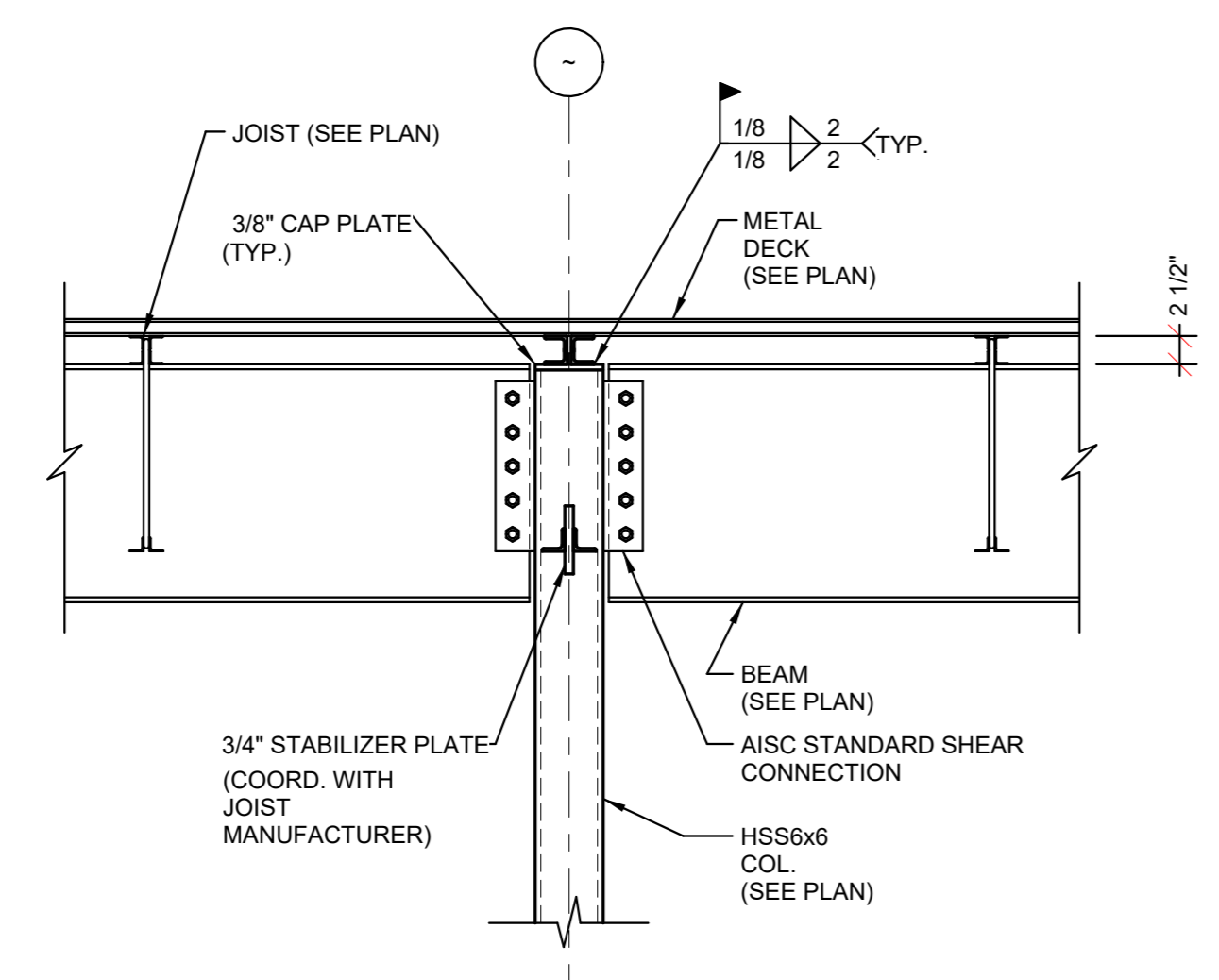
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FRAMING
DETAILS

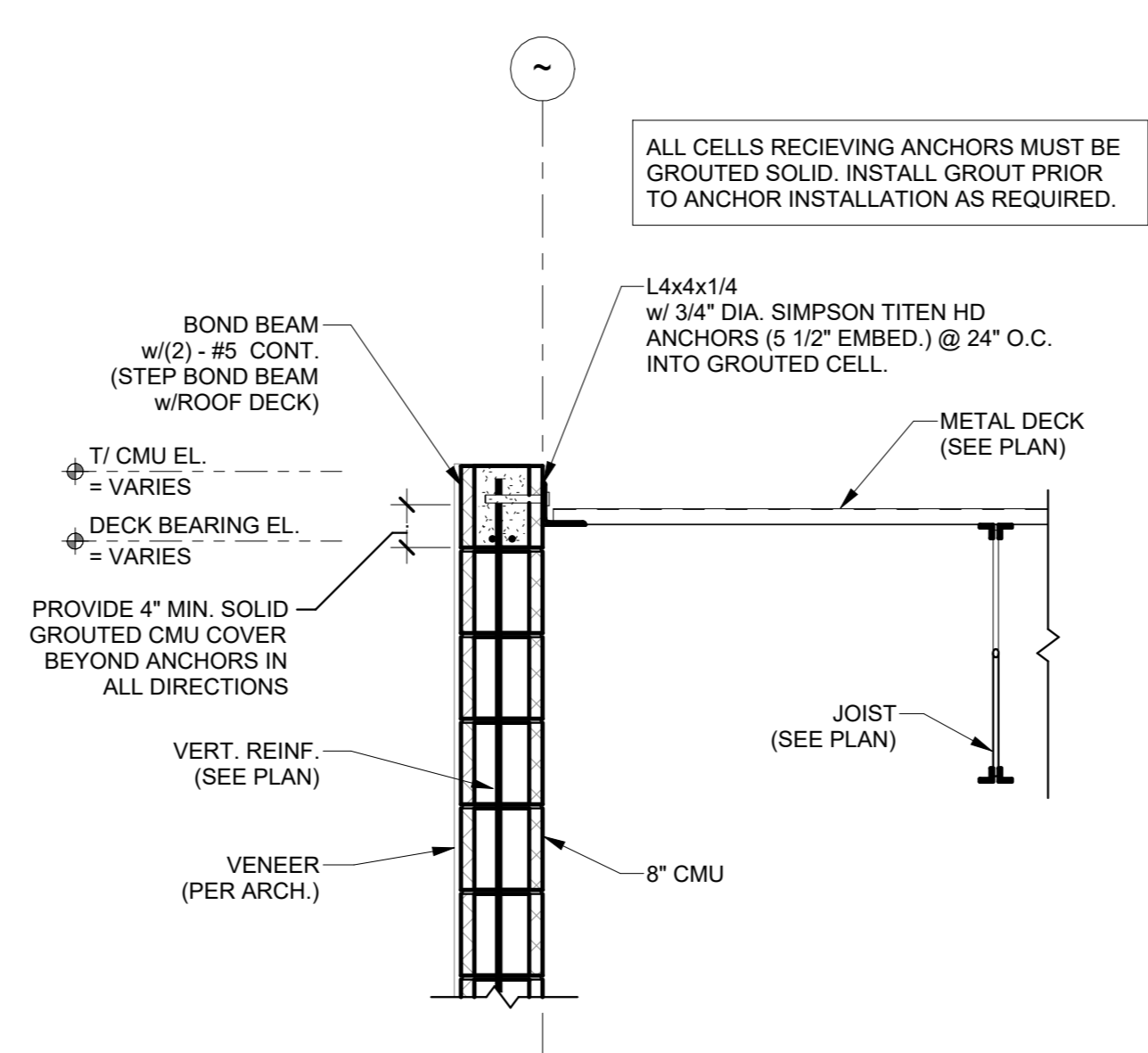
S4.0



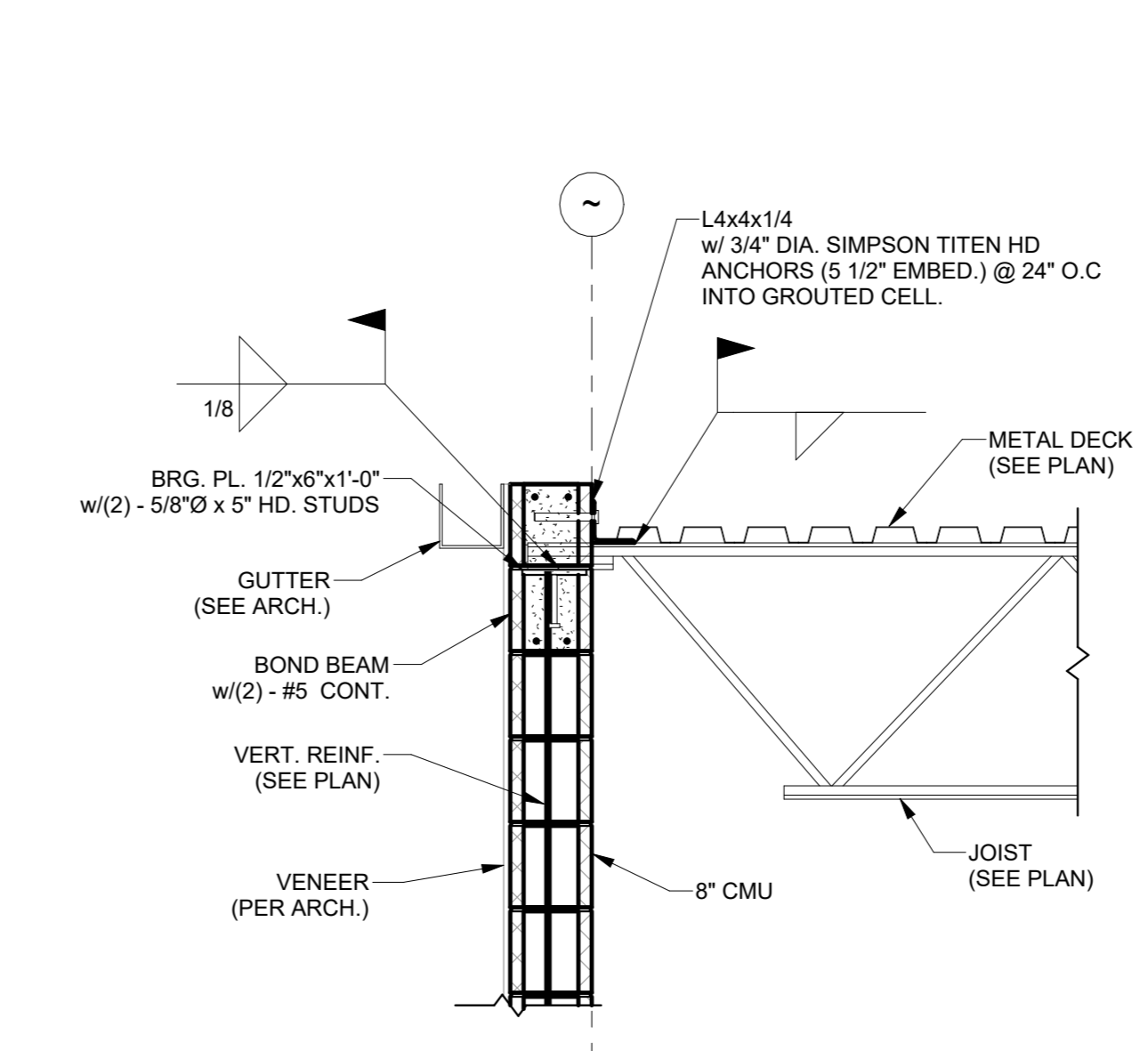
1 JOIST BEARING DETAIL
SCALE: N.T.S.



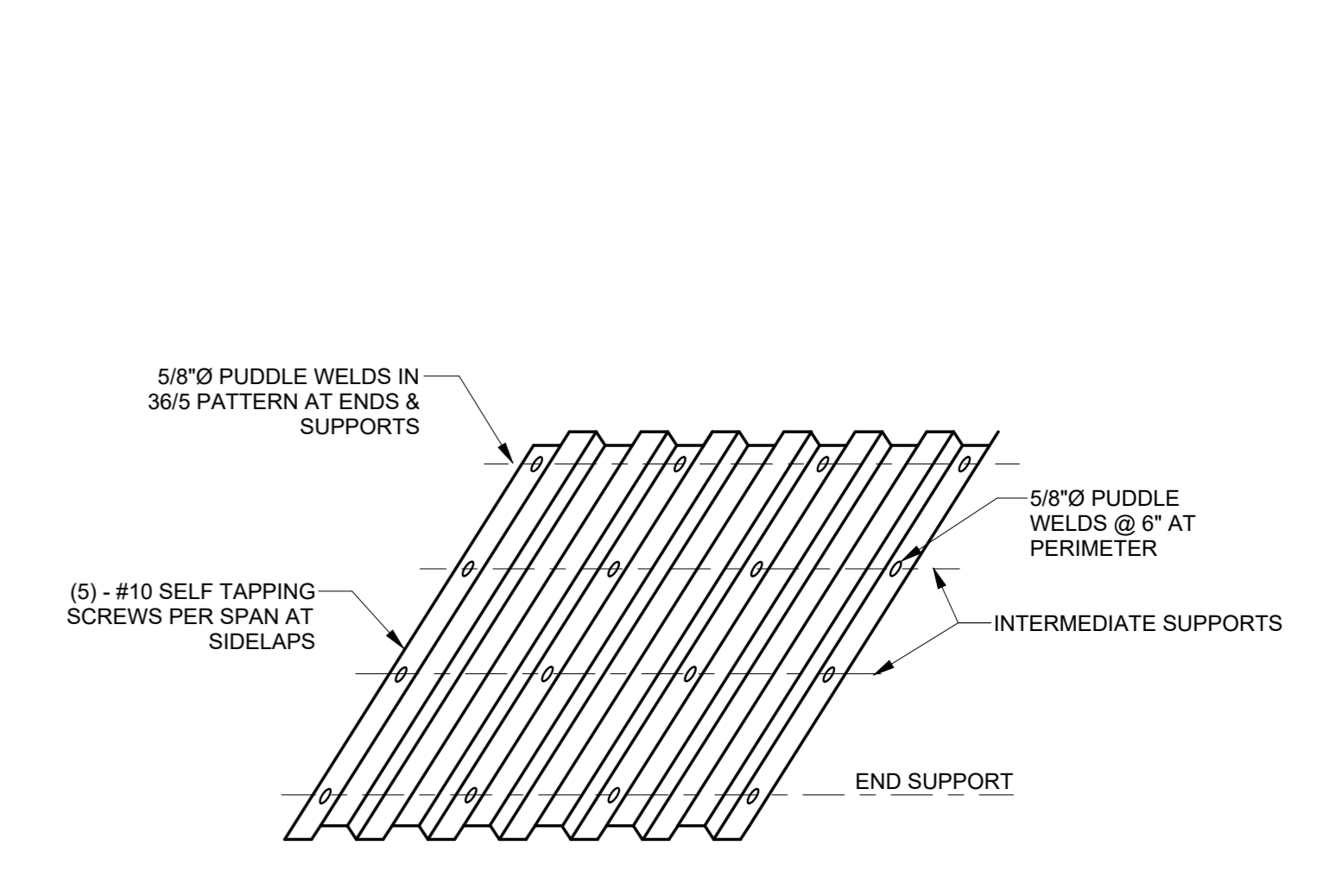
2 JOIST BEAM-TO-COLUMN CONNECTION
SCALE: 3/4\"/>



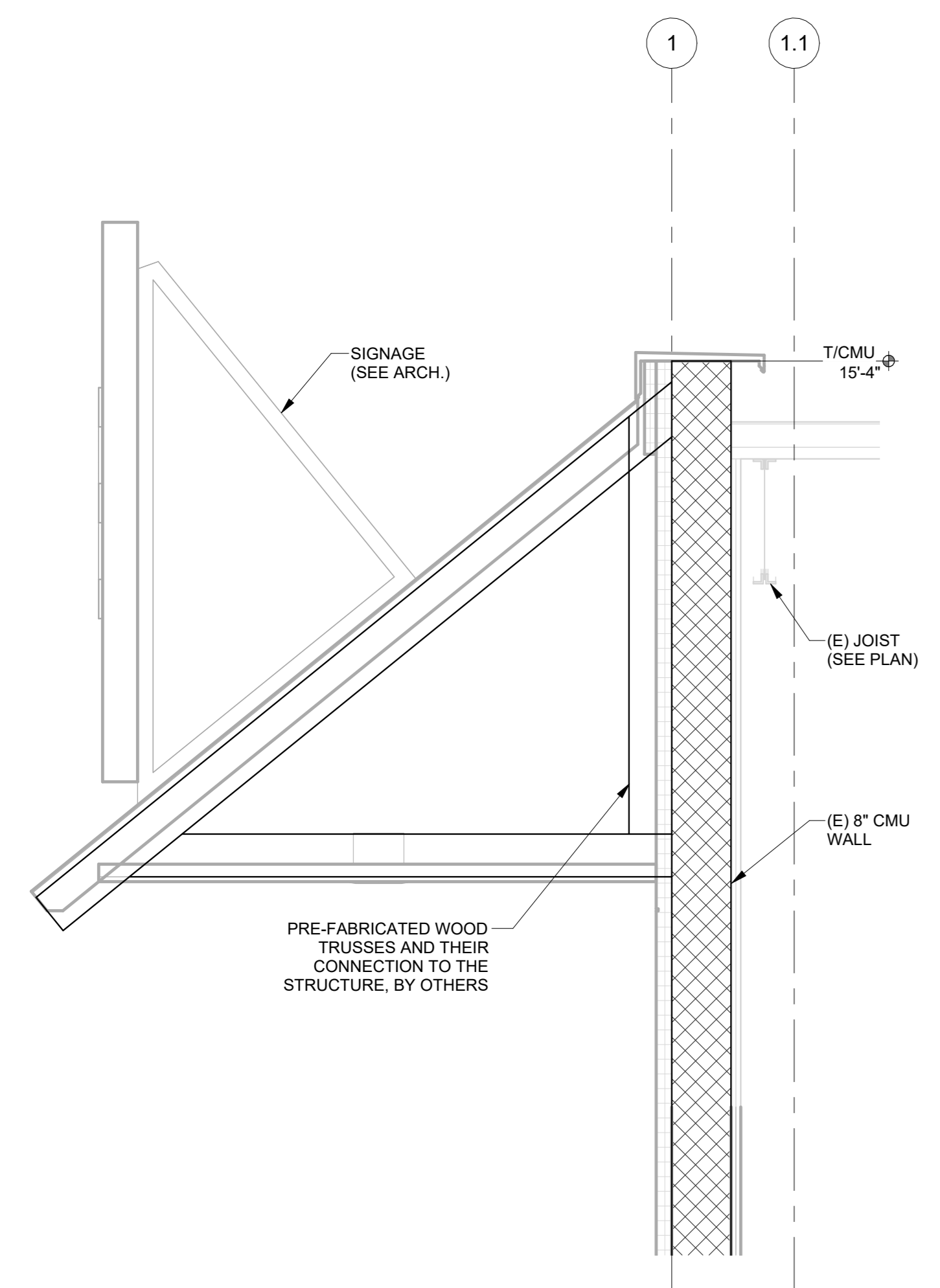
3 TYP. DECK BEARING AT CMU
SCALE: 3/4\"/>



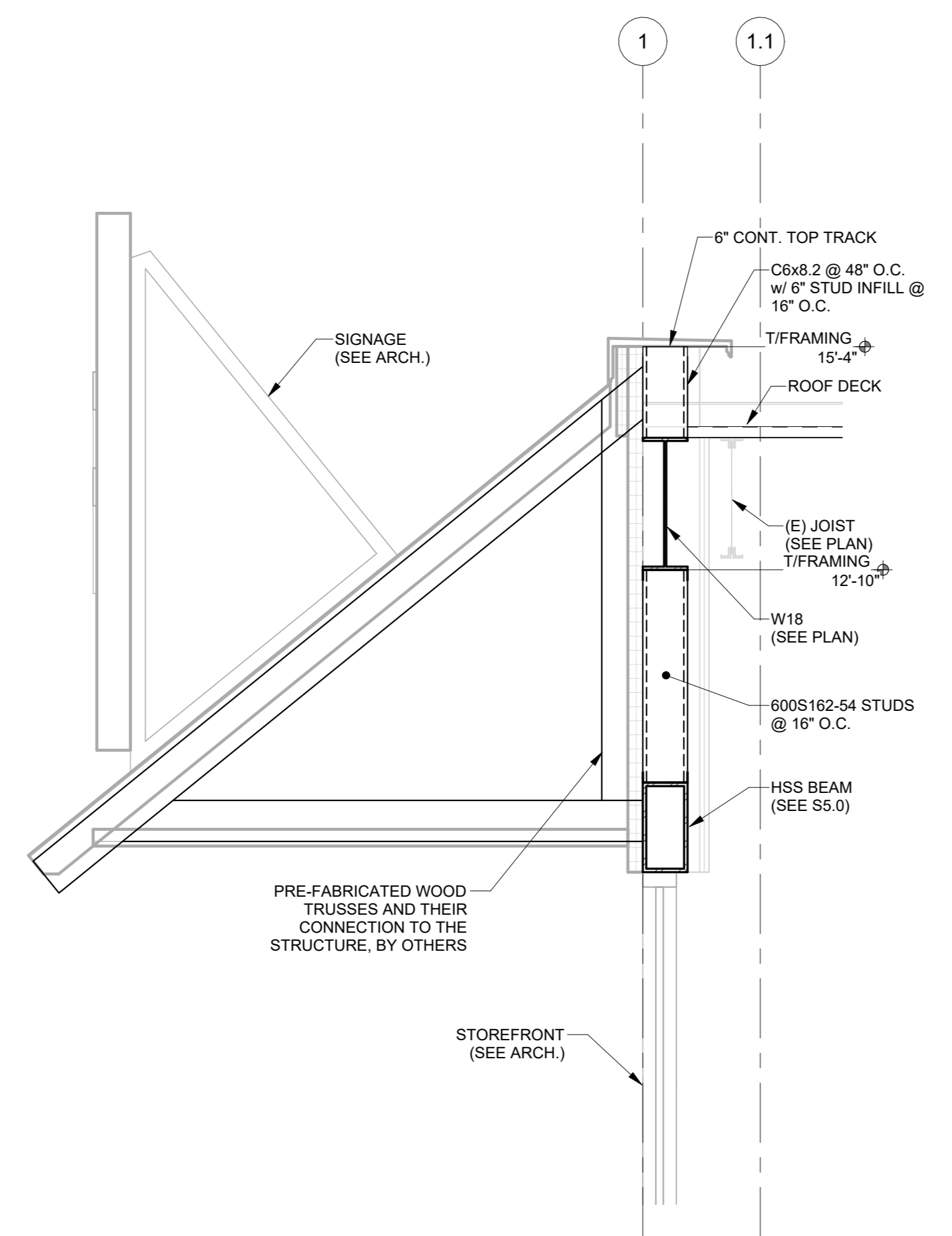
4 TYP. JOIST BEARING AT CMU
SCALE: 3/4\"/>



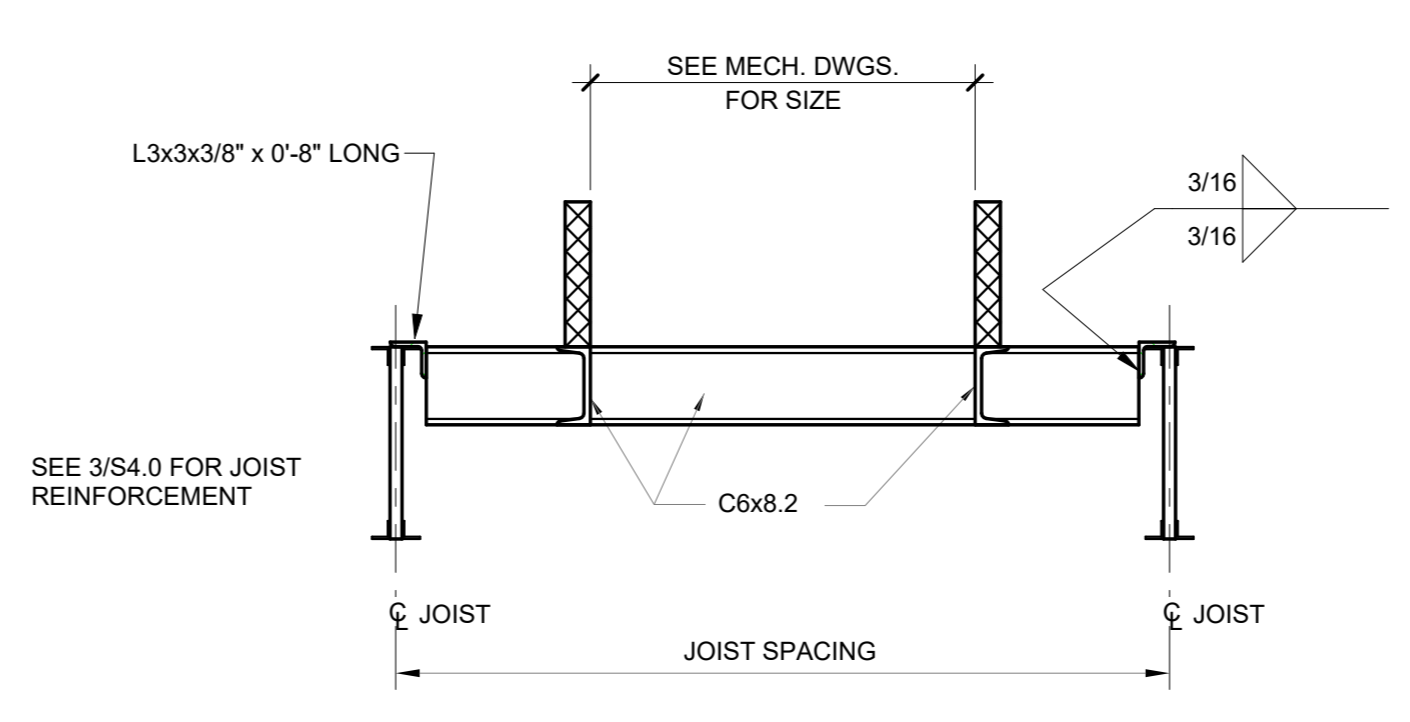
5 STEEL ROOF DECK ATTACHMENT DETAIL
SCALE: N.T.S.



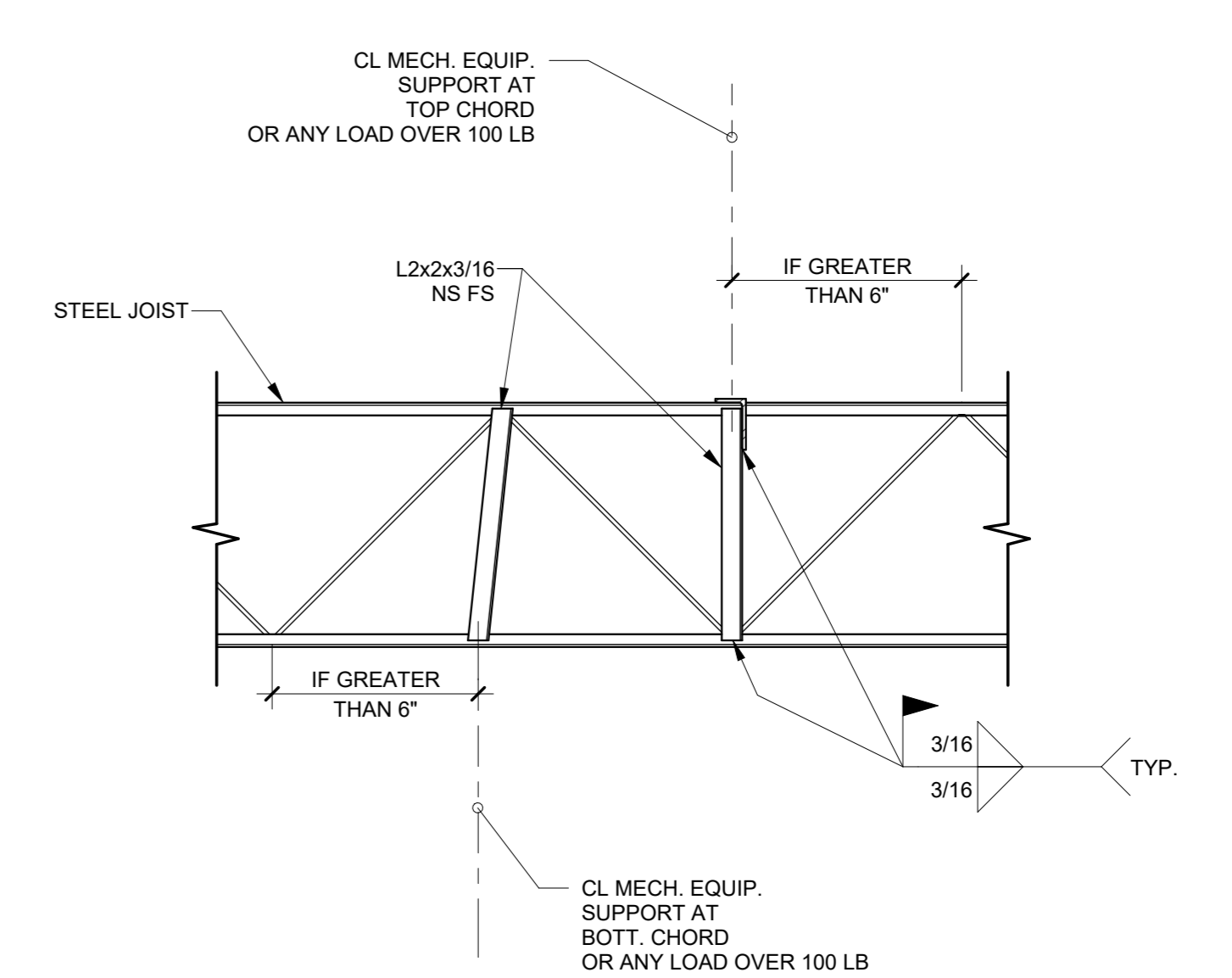
6 SECTION
SCALE: 3/4\"/>



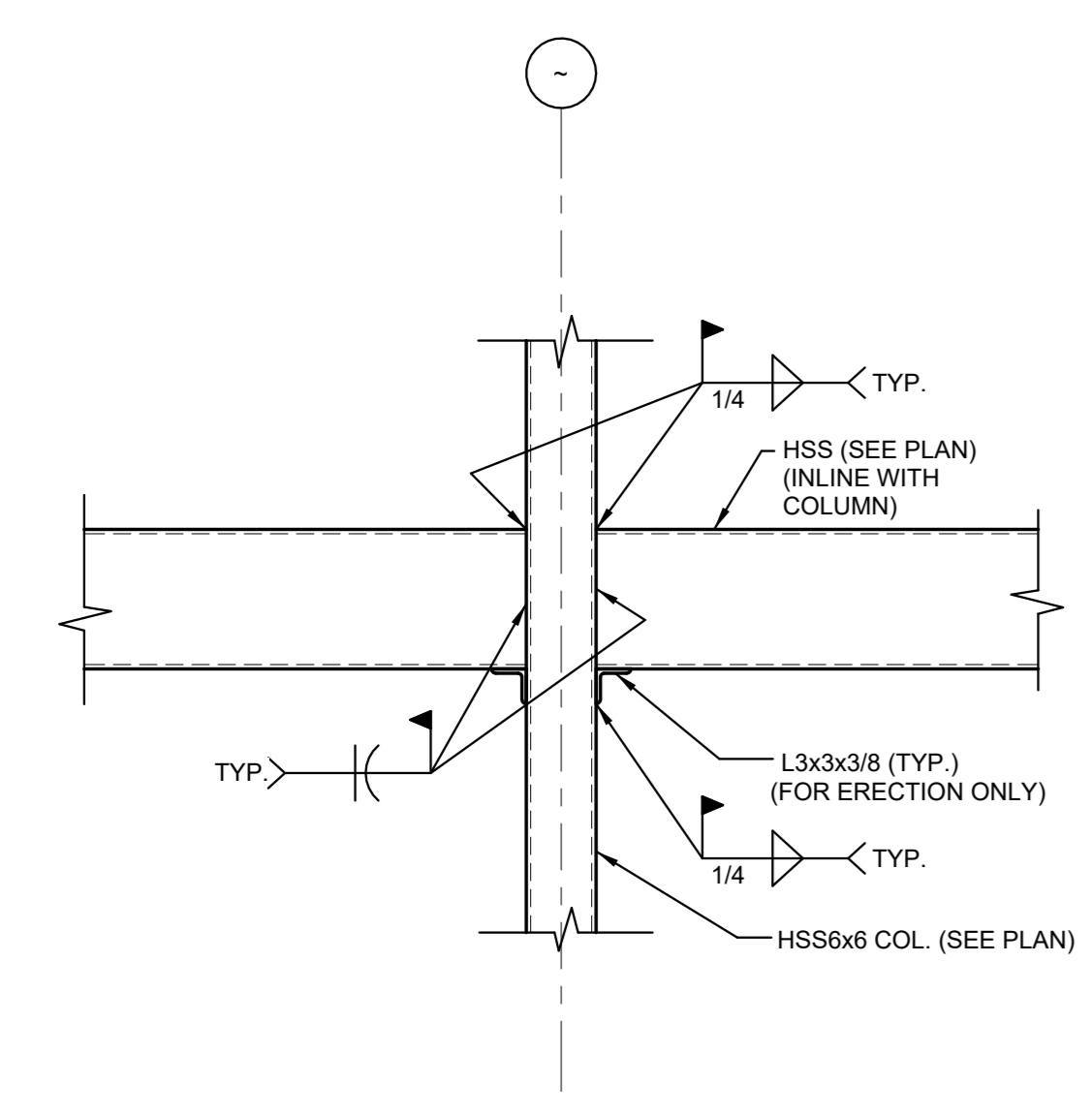
7 SECTION
SCALE: 3/4\"/>



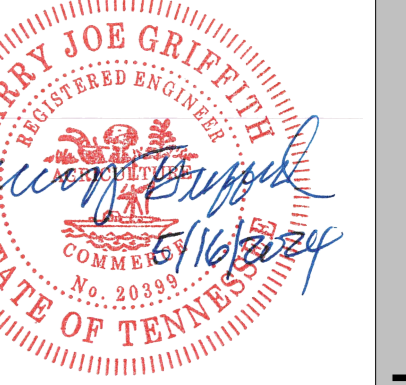
8 TYPICAL MECH. UNIT CURB SUPPORT
SCALE: 3/4\"/>



9 JOIST REINFORCEMENT DETAIL FOR NON-PANEL POINT LOADING
SCALE: 3/4\"/>



10 HEADER BEAM-TO-COLUMN CONNECTION
SCALE: 3/4\"/>



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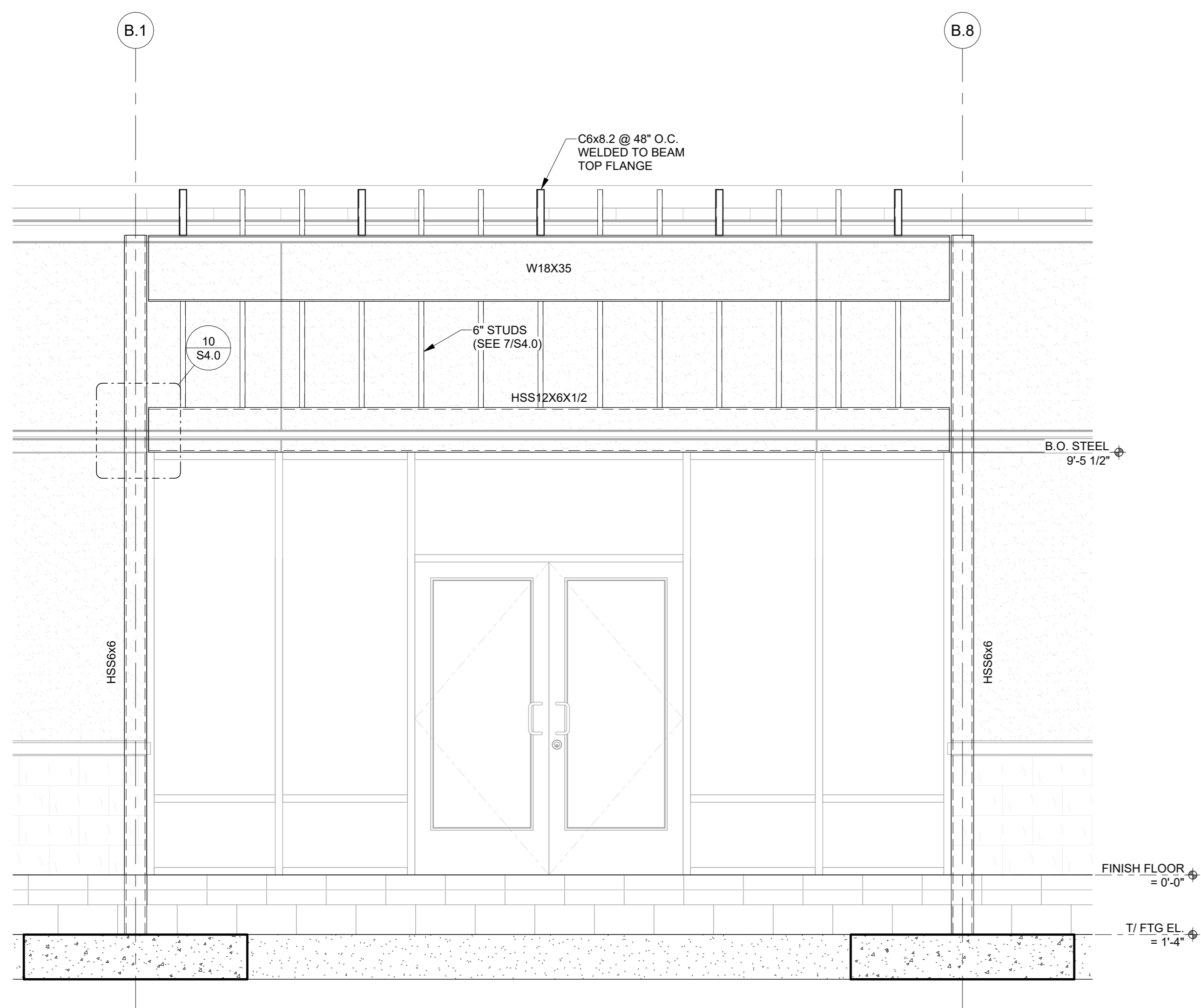
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FRAMING ELEVATIONS

SHEET

S5.0



1 FRAMING ELEVATION ALONG GRID LINE "1"
 SCALE: 1/2" = 1'-0"



Caleb T. Weaver
P.E.

SUITE 5204 479-636-0004
1805 N 2ND ST JOB NO.: 31048
ROSELAND, AR 72766 DESIGNED BY: T.L.B.



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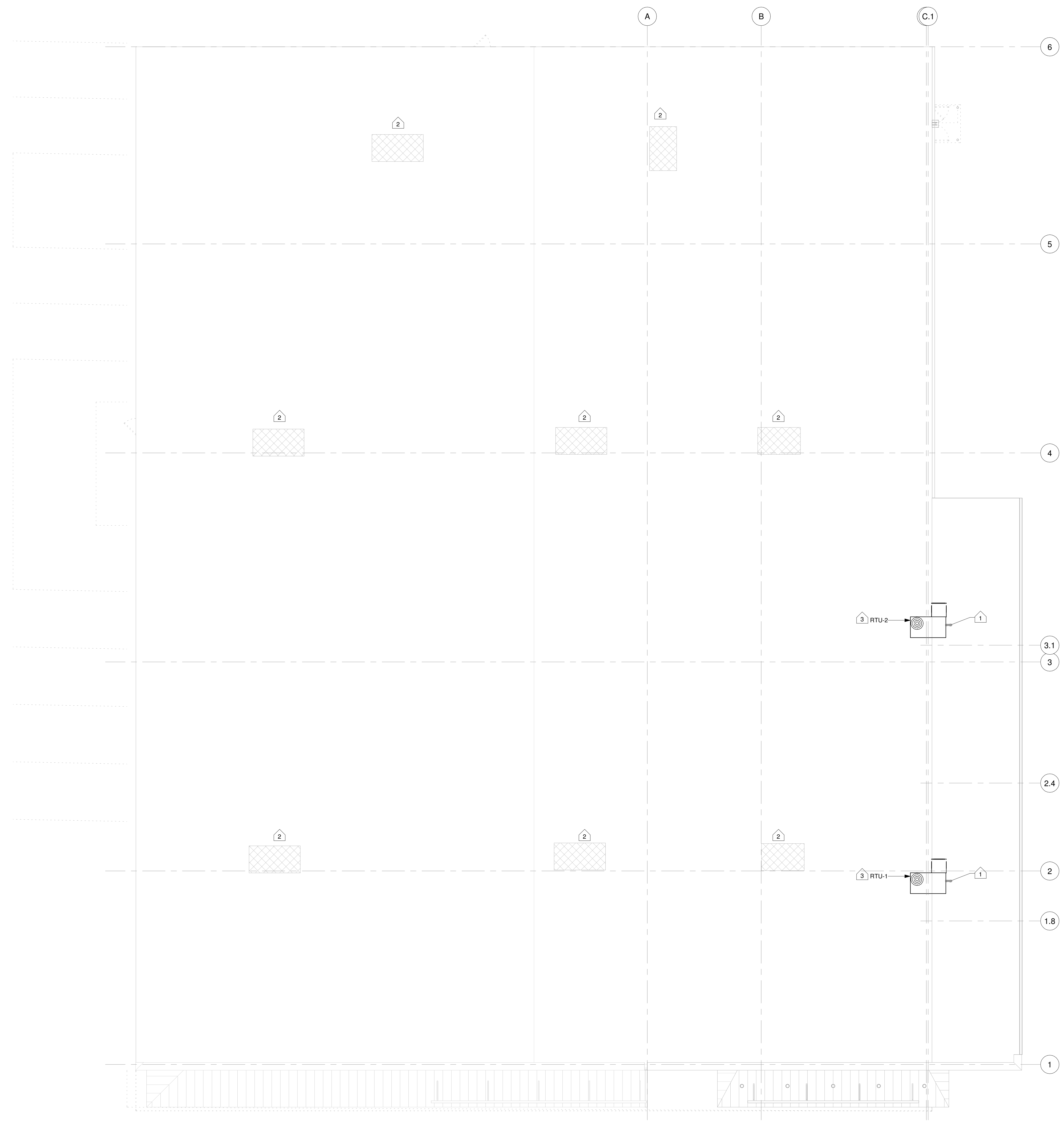
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MECHANICAL
DESIGN PLAN

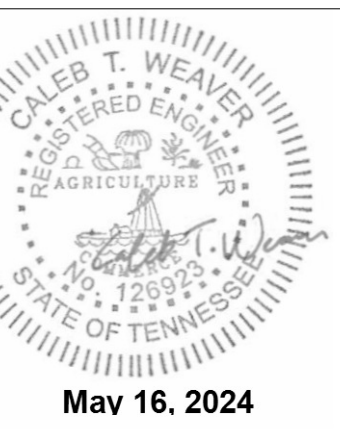
M100

KEYED NOTES

1. PROVIDE PVC P-TRAP FOR CONDENSATE. SIZE P-TRAP EQUAL TO CONNECTION SIZE. DRAINS TO TERMINATE PER LOCAL A.H.J. TYPICAL.
2. EXISTING ROOFTOP UNIT TO REMAIN. SHOWN FOR REFERENCE ONLY.
3. PROVIDE MINIMUM 150 FEET OF LOW VOLTAGE THERMOSTAT CABLE WIRED TO UNITS. LOOPED AND LEFT HANGING FROM STRUCTURE. PROVIDE WITH PROGRAMMABLE THERMOSTAT AND REMOTE SENSOR FOR TENANT.



1 ROOF PLAN
SCALE: 1/8" = 1'-0"



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OUTSIDE AIR CALCULATION - "OSA"											
DESIGN ID	AREA SERVED	OCCUPANCY CATEGORY	FLOOR AREA (Az)	ZONE POPULATION (Pz)	OUTDOOR AIR CFM PER PERSON (Rp)	OUTDOOR AIR CFM PER SF (Ra)	ZONE AIR EFFECTIVENESS (Ez)	CALCULATED MINIMUM (CFM)	DESIGN MINIMUM (CFM)	CALCULATED DCV MINIMUM (CFM)	DCV MINIMUM (CFM)
RTU-1,2	RETAIL	RETAIL SALES	1502	22	7.50	0.12	0.8	432	435	225	225

ROOF TOP UNIT (RTU) SCHEDULE																						
DESIGN ID	SERVES	NOMINAL TONS	MANUFACTURER	MODEL	CFM	OUTSIDE AIR			COOLING CAPACITY			HEAT PUMP CAPACITY MBH AT 17 F	HEATING CAPACITY			WEIGHT - LBS	NOTES					
						DESIGN MIN	DCV MIN	ESP	TOTAL SENSIBLE - MBH	EDB	EWB		kw INPUT	EQ MBH	SEER			EER	HSPF	VOLTAGES	MCA	MOCp
RTU-1	SALES	3	CARRIER	50HCOA04	1200	220	115	1.0	36.7 / 28.2	80°F	67°F	16.9	7.9	26.96	15.6	12.7	8.0	208/360	46.6	50	780	1 THRU 10
RTU-2	SALES	3	CARRIER	50HCOA04	1200	215	110	1.0	36.7 / 28.2	80°F	67°F	16.9	7.9	26.96	15.6	12.7	8.0	208/360	46.6	50	780	1 THRU 10

- NOTES:**
- FULL PERMETER INSULATED ROOF CURB-ADJUSTABLE.
 - FULLY MODULATING DIFFERENTIAL ENTHALPY ECONOMIZER.
 - FACTORY NON-POWERED GFCI RECEPTACLE.
 - FACTORY DISCONNECT WITH FUSING.
 - NEW HEAT PUMP-ELECTRIC COOLING/ELECTRIC HEATING ROOFTOP PACKAGED UNIT. VERIFY FINAL LOCATIONS AT JOBSITE.
 - HIGH EFFICIENCY, DOWN DISCHARGE CONFIGURATION.
 - TENANT PROVIDED EMS CONTROLS PACKAGE.
 - PROVIDE WITH HAIL GUARDS.
 - PROVIDE WITH BAROMETRIC RELIEF.
 - TENANT PROVIDED DEMAND CONTROL VENTILATION AND CO2 SENSOR.

GENERAL MECHANICAL NOTES AND SPECIFICATIONS

DESCRIPTION OF WORK:

The work under this section includes everything necessary for and incidental to executing and completing the heating, ventilating and air conditioning work except, as hereafter specifically excluded.

SCOPE:

Furnish material and perform labor required to execute the work of this section as specified herein and necessary to complete the Contract, including, but not limited to, the following principal items:

- Air conditioning system with filters.
- Automatic controls, electric type.
- Permits, plan check, and fees.

WORK BY OTHERS:

- The following index of work is included for clarification only.
- Work to be performed in accordance with this section of the specifications includes, but is not limited to:
 - Condensate drains and gas lines by Plumbing Contractor.
 - Sheetmetal equipment platform covers by General Contractor.
 - Burglar bars (if required) by General Contractor.
 - Furnish all magnetic starters, relays, control devices, etc., required for air conditioning equipment. Time clocks and bypass timers (when required) furnished by Electrical Contractor.
 - Furnish and install all line and low voltage wiring and conduit for air conditioning equipment by Electrical Contractor.
 - Supply and installation of Energy Management or Building Automation System if/as required by these Specifications.

SUBMITTALS:

- PRE-CONSTRUCTION SUBMITTALS: Within 14 days after award of contract, and BEFORE any equipment is ordered, submit 6 BOUND and INDEXED SUBMITTAL BOOKLETS which shall include the following:
 - AIR CONDITIONING SHOP DRAWINGS:
 - The shop drawings shall include equipment and elevations and specific, dimensioned details of all equipment mounting and supports, all piping and equipment appurtenances and counterflashings required for the installation of the systems.
 - The ductwork details shall include duct plans and elevations, dimensioned and detailed sheetmetal connectors, locations and gauges.
 - EQUIPMENT SPEC AND PRODUCT INFORMATION SUBMITTAL BOOKLETS:
 - BOUND and INDEXED copies of all manufacturer's product data shall be submitted in a package with the shop drawing requirement above. Only a complete submittal shall be approved. For each item, the brochures must be clearly marked to indicate the size, model number, accessories, options, electrical characteristics, velocities, sheet metal gauges, dimensions, and operating weights.
 - ANY SUBSTITUTIONS MUST BE CLEARLY INDICATED AS SUCH.
 - The submittal brochure shall include:
 - AIR CONDITIONING UNITS - See Section 3.1.
 - THERMOSTATS.
 - FILTER SIZES as recommended by the unit manufacturer.
 - Unit filters shall be min Merv 8, per section 5.504.53 CAL Green Code.
- POST-CONSTRUCTION SUBMITTALS: Upon completion of work, the Contractor shall submit the following complete package to the Architect for approval:
 - AS-BUILT DRAWINGS:
 - Maintain throughout the progress of the work a detailed set of as-built drawings reflecting ANY changes to the original engineered plan. Changes shall include but are not limited to duct locations, duct sizes, equipment changes, tenant improvement changes, field change orders, and local code corrections. Upon completion, submit to the Architect a set of reversed reproducible prints for approval.
 - OPERATING AND MAINTENANCE MANUALS: Submit four (4) copies of all operating instructions and maintenance manuals as provided by each equipment manufacturer to the Architect for approval.

PACKAGED, OUTDOOR, CENTRAL-STATION AIR-HANDLING UNITS:

- This Section includes packaged, outdoor, central-station air-handling units (rooftop units) with the following components and accessories:
 - Direct-expansion cooling.
 - Electric-heating coils.
 - Heat pump heating with electric supplemental heating.
 - Economizer outdoor- and return-air damper section.
 - Integral space temperature controls.
 - Roof curbs.

DEFINITIONS:

- Outdoor-Air Refrigerant Coil: Refrigerant coil in the outdoor-air stream to reject heat during cooling operations and to absorb heat during heating operations. "Outdoor air" is defined as the air outside the building or taken from outdoors and not previously circulated through the system.
- Outdoor-Air Refrigerant-Coil Fan: The outdoor-air refrigerant-coil fan in RTUs. "Outdoor air" is defined as the air outside the building or taken from outdoors and not previously circulated through the system.
- RTU: Rooftop unit. As used in this Section, this abbreviation means packaged, outdoor, central-station air-handling units. This abbreviation is used regardless of whether the unit is mounted on the roof or on a concrete base on ground.
- Supply-Air Fan: The fan providing supply-air to conditioned space. "Supply air" is defined as the air entering a space from air-conditioning, heating, or ventilating apparatus.
- Supply-Air Refrigerant Coil: Refrigerant coil in the supply-air stream to absorb heat (provide cooling) during cooling operations and to reject heat (provide heating) during heating operations. "Supply air" is defined as the air entering a space from air-conditioning, heating, or ventilating apparatus.

QUALITY ASSURANCE:

- ARI Compliance:
 - Comply with ARI 203/110 and ARI 303/110 for testing and rating energy efficiencies for RTUs.
 - Comply with ARI 270 for testing and rating sound performance for RTUs.
- ASHRAE Compliance:
 - Comply with ASHRAE 15 for refrigerant system safety.
 - Comply with ASHRAE 53 for methods of testing cooling and heating coils.
- ASHRAE/IESNA 90.1 Compliance: Applicable requirements in ASHRAE/IESNA 90.1, Section 6 - "Heating, Ventilating, and Air-Conditioning."
- NFPA Compliance: Comply with NFPA 90A and NFPA 90B.
- UL Compliance: Comply with UL 1995.
- Electrical Components, Devices, and Accessories: Listed and labeled as defined in NFPA 70, Article 100, by a testing agency acceptable to authorities having jurisdiction, and marked for intended use.

EXECUTION:

1.1 INSPECTIONS:

It shall be the Contractor's responsibility to contact the Inspector at the Mechanical Engineering Consultant, one week prior to the covering of any work for the purposes of inspecting the work. The Contractor shall provide easy access to the installation above and below the roof. In the event that the Contractor fails to notify in time, the Contractor shall bear all costs incurred to obtain required access for inspection.

REINSPECTIONS:

If any materials or workmanship fails to meet the requirements of the plans and specifications and the inspection, a reinspection shall be required after correction at the cost of two and a half (2-1/2) times the direct expense of the inspector. The same access requirements at the initial inspection shall hold.

1.2 EQUIPMENT INSTALLATION AND LOCATION:

- All locations shall be approved by the project structural engineer through the Architect prior to roof equipment layout.
- Outdoor part of the air conditioning units shall be located on the roof. Units shall be firmly secured to avoid displacement. Ductwork shall be located below the roof.

1.3 ACCESSIBILITY:

- Install all dampers and other accessories, in convenient and accessible locations with reference to the finished building.
- Furnish access doors and panels in sheet metal duct or plenums as necessary for the operation of the mechanical equipment.

1.4 DAMPERS:

- Install approved fire dampers at all locations where required by code. Provide access panels as necessary to permit damper and fusible link accessibility.
- Install a sufficient number of volume dampers to properly balance the fresh air, return air and supply air, of each main and each branch duct. Provide OSA and RA dampers at plenums.

1.5 NOISE AND VIBRATION:

- Complete system to be free of objectionable noise and vibration in the opinion of the Architect.
- Be prepared to make proper provisions, provide all material and pay all costs necessary to remove objectionable noise and vibration.

1.6 TESTING AND ADJUSTING:

- Furnish all required personnel and equipment, and perform all tests required to secure approval of the installation from all agencies having jurisdiction. Make all necessary control adjustments and balancing of air flow per SMACNA Standards, and then operate entire system for purpose of providing satisfactory performance. During this period, instruct all responsible persons that Architect will designate in proper operation, care and maintenance of system.
- Adjust or replace fan shaves, register dampers, proportioning vanes, and directional devices to obtain uniform air distribution of design quantities shown.

1.7 OPERATING INSTRUCTIONS:

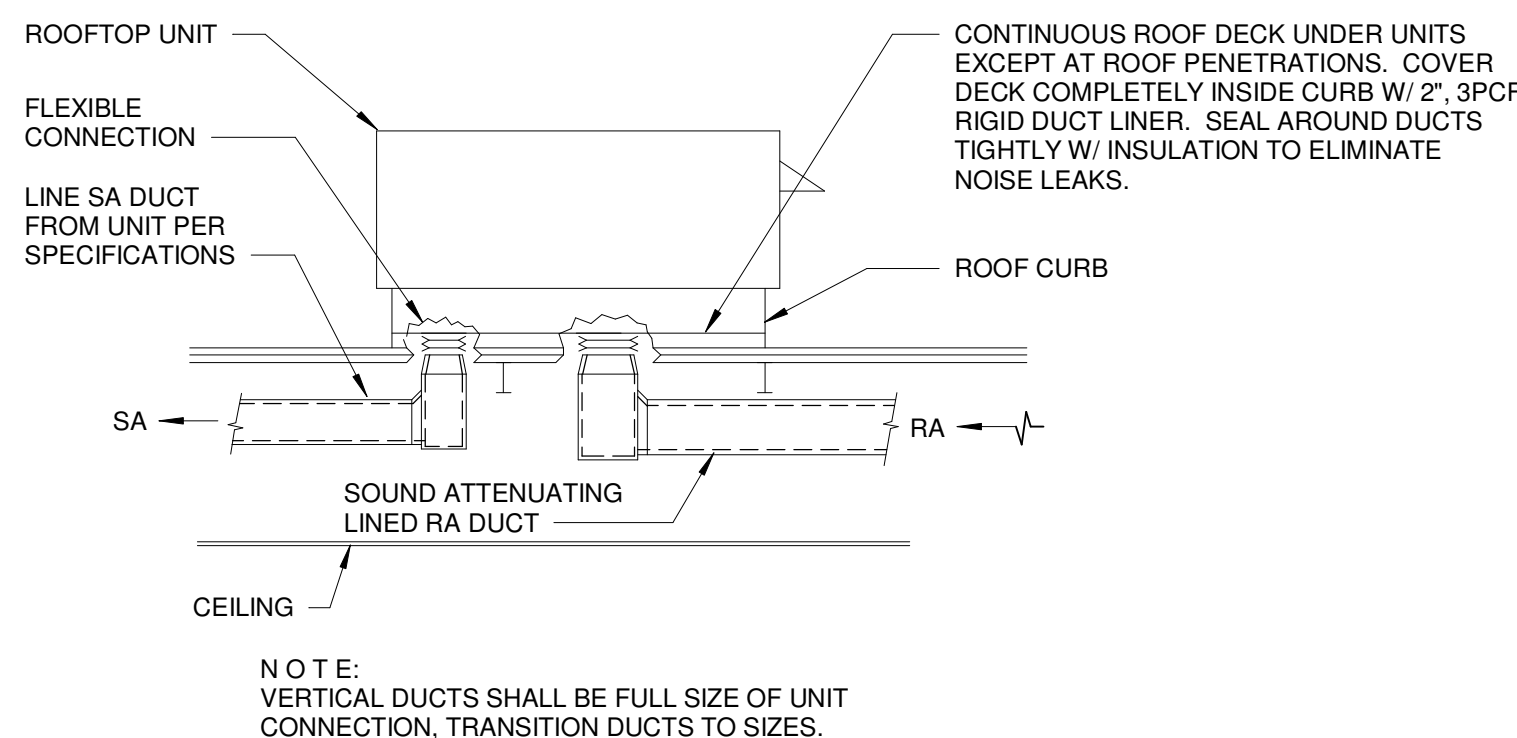
- Upon completion of the work, the Contractor shall furnish the Architect and Owner with a complete description of the equipment and operation of each system, the Air Balance Report and also a list of equipment requiring maintenance, periodic oiling, filter changes etc.

1.8 GUARANTEES

- All apparatus and equipment furnished and installed under the contract shall be new and free from all defects and shall be guaranteed for a period of one (1) year from the date of acceptance of the building, compressors shall have full five (5) year factory warranty. Should any trouble develop during this period due to inferior or defective materials or faulty workmanship, the Contractor shall correct the trouble without cost to the Owner. He shall guarantee all apparatus to operate in accordance with the specifications covering each item. Should this Contractor refuse or neglect to make any test necessary to satisfy the Architect that he has carried out the true intent and meaning of the specifications, the Owner may make such tests and charge the expense thereof to this Contractor.

SEISMIC REQUIREMENTS (AS APPLICABLE):

- FURNISH AND INSTALL SEISMIC BRACING FOR MECHANICAL COMPONENTS. RESTRAINTS SHALL BE DESIGNED BASED ON SEISMIC DESIGN CATEGORY REQUIREMENTS AND PER SMACNA PUBLISHED SEISMIC DETAILS, SEISMIC RESTRAINT MANUFACTURER'S DETAILS AND LOCAL AND NATIONAL CODES. CONTRACTOR'S RESPONSIBILITY INCLUDES STRUCTURAL ENGINEER'S CERTIFICATION ON DETAILS SUBMITTED FOR PERMITTING.
- THE GENERAL CONTRACTOR IS TO SECURE THE SERVICES OF A SEISMIC RESTRAINT MANUFACTURER'S ENGINEER TO DESIGN THE SEISMIC RESTRAINT SYSTEMS FOR ALL MECHANICAL COMPONENTS.
- SUBMIT A LETTER BEARING THE SEAL OF THE DESIGNER OF THE SEISMIC PLAN TO THE A/E'S OFFICE VERIFYING THE INSTALLATION AS PERFORMED IN THE FIELD MEETS THEIR INTENT AND IS ACCEPTABLE IN THEIR PROFESSIONAL VIEW PRIOR TO THE 1ST MECHANICAL INSPECTION, CONCEALMENT OF ANY PORTION OF THE SYSTEM, OR ANY SUBSEQUENT INSPECTION.



1 ROOF TOP UNIT DETAIL
 SCALE: N.T.S.



Scott C. Odom
P.E.

SUITE 5530 479-636-5004
1805 N 2ND ST JOB NO: 21046
ROCKERS, AK 72756 DESIGNED BY: NWY



FLETCHER BRIGHT
COMPANY

1007 ASHLAND TERRACE
SUITE 104A
CHATTANOOGA, TN 37415

DOLLAR TREE
SHELL -
McMINNVILLE

912 N CHANCERY ST
McMINNVILLE, TN 37110
WARREN COUNTY

PERMIT SET
05/16/24
MJM # 23336

NO.	DESCRIPTION	DATE

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POWER PLAN

E-100

NOT FOR CONSTRUCTION NOT FOR CONSTRUCTION NOT FOR CONSTRUCTION NOT FOR CONSTRUCTION NOT FOR CONSTRUCTION

SEAL

CONSULTANT

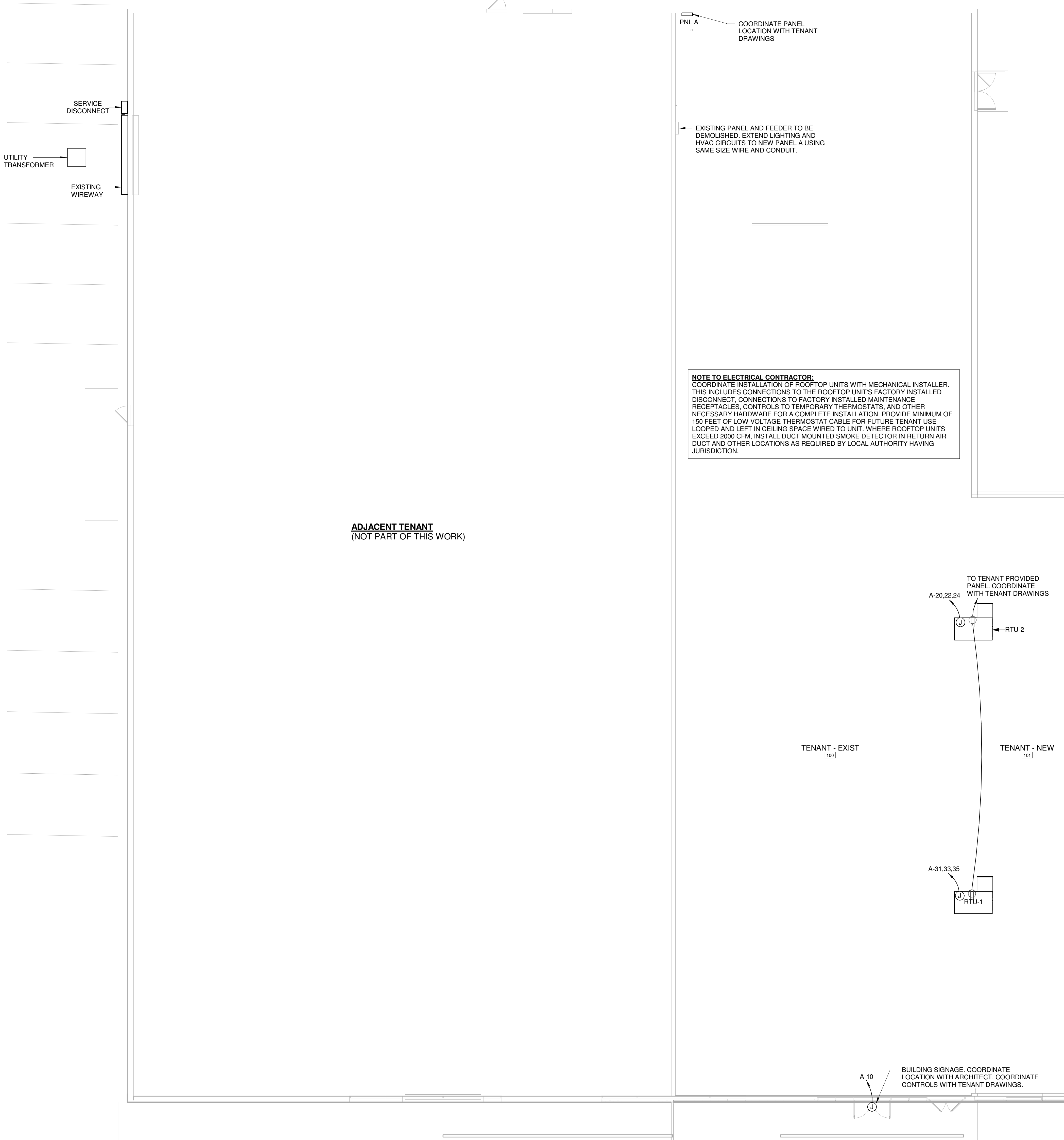
OWNER

LOCATION

DATE

REVISIONS

SHEET

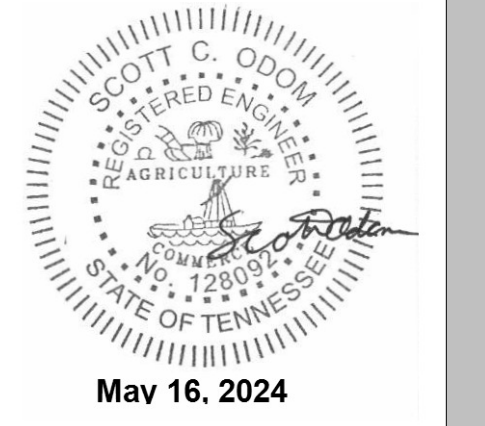


LIGHTING FIXTURE SCHEDULE					
TYPE	DESCRIPTION	VOLTAGE	LOAD	MANUFACTURER	MODEL
A	5 7/8" ROUND LED, WHITE FINISH WHITE FLANGE	120 V	18 VA	LITHONIA LIGHTING	LDN6 35K 15 L06 AR LSS TRW MVOLT GZ10
B	ARCHITECTURAL WALL SCONCE	120 V	25 VA	LITHONIA LIGHTING	WST LED P2 30K VF MVOLT E7WC
C	4FT LINEAR LED	120 V	44 VA	LITHONIA LIGHTING	CSS L48 ALD3 MVOLT SWW3 80CRI
EM	LED EMERGENCY LIGHT DOUBLE - WHITE	120 V	5 VA	LITHONIA LIGHTING	ELM4L
EX	EXIT SIGN WITH EMERGENCY LIGHT - WHITE AND RED	120 V	4 VA	LITHONIA LIGHTING	EGRG HO SQ M6
RH	LED EMERGENCY LIGHT DOUBLE - WHITE	120 V	3 VA	LITHONIA LIGHTING	AFB OELR WT

GENERAL LIGHTING NOTES
 A. CENTER EMERGENCY LIGHTS / EXIT SIGNS ABOVE DOORS UNLESS NOTED OTHERWISE.



ARCHITECTURE
 PLANNING
 INTERIORS
 GRAPHICS



Scott C. Odom
 P.E.

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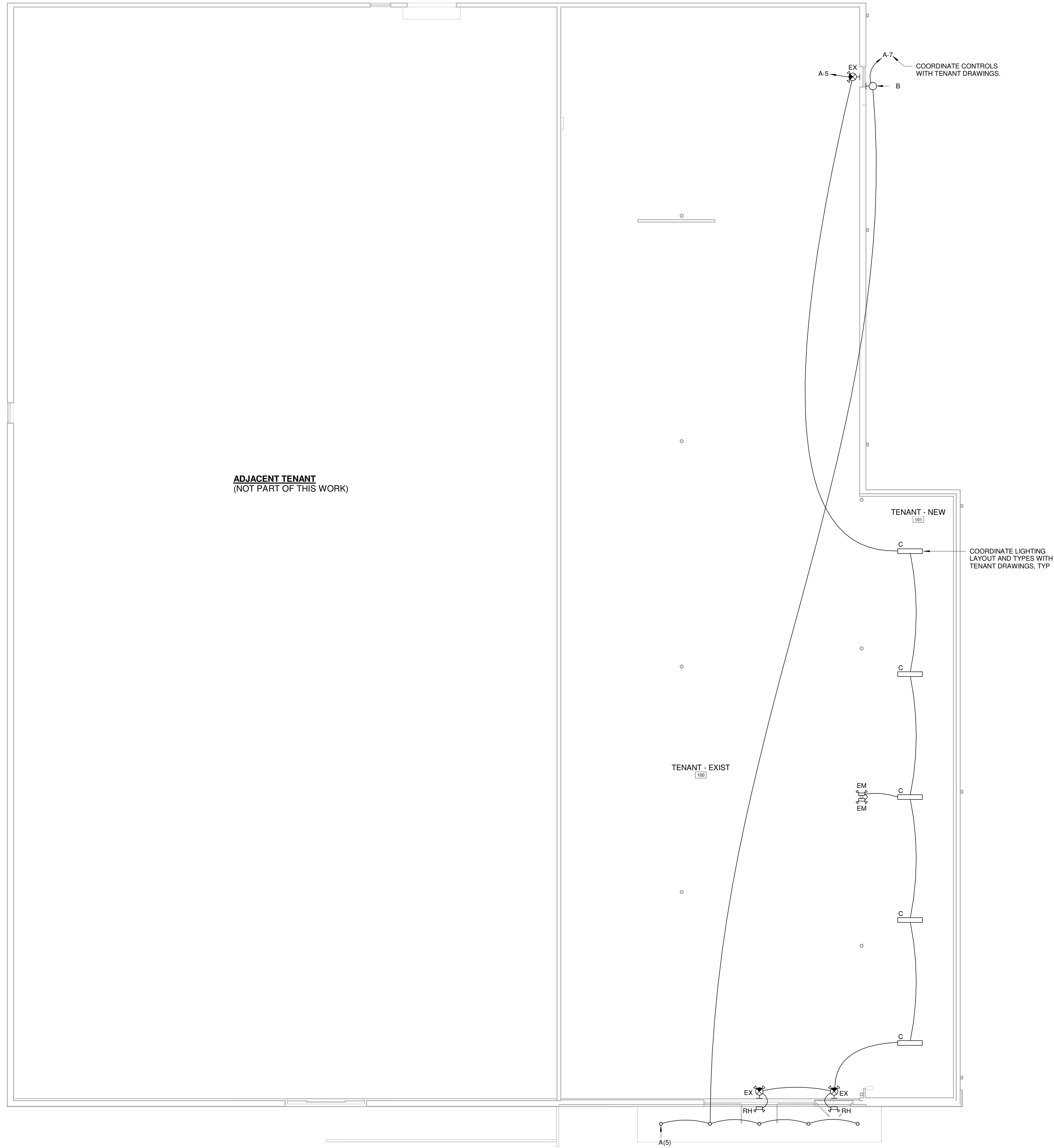
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LIGHTING PLAN

E-200



ADJACENT TENANT
 (NOT PART OF THIS WORK)

TENANT - NEW
 100

TENANT - EXIST
 100

COORDINATE LIGHTING
 LAYOUT AND TYPES WITH
 TENANT DRAWINGS, TYP

COORDINATE CONTROLS
 WITH TENANT DRAWINGS.

NOT FOR CONSTRUCTION

ARCHITECT SEAL CONSULTANT OWNER LOCATION DATE REVISIONS SHEET

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